

Town of Truro Zoning Board of Appeals P.O. Box 2030, Truro, MA 02666

APPLICATION FOR HEARING

| To the Town Clerk of the Town of Truro, MA The undersigned hereby files with specific grounds for this application: (check all that apply) |
|--|
| GENERAL INFORMATION |
| □ NOTICE OF APPEAL |
| ☐ Applicant is aggrieved by his/her inability to obtain a permit or enforcement action from the Building |
| Commissioner on (date) |
| Applicant is aggrieved by order or decision of the Building Commissioner on <i>(date)</i> which he/she believes to be a violation of the Truro Zoning Bylaw or the <i>Massachusetts Zoning Act</i> . |
| □ PETITION FOR VARIANCE – Applicant requests a variance from the terms § of the Truro |
| Zoning Bylaw concerning (describe) |
| |
| ☑ APPLICATION FOR SPECIAL PERMIT |
| Applicant seeks approval and authorization of uses under § 50.1-B of the Truro Zoning Bylaw |
| concerning (describe) height limitations |
| _ |
| Applicant seeks approval for a continuation, change, or extension of a nonconforming structure or use |
| under § of the Truro Zoning Bylaw and M.G.L. Ch. 40A, §6 concerning (describe) |
| 2000 IN 11 12 Co. H. O. I |
| Property Address 113 Castle Road Map(s) and Parcel(s) 46-398 |
| Registry of Deeds title reference: Book, Page, or Certificate of Title |
| Number and Land Ct. Lot # and Plan # |
| Applicant's Name Any Holt |
| Applicant's Legal Mailing Address 15 Andrew St. Newton, MA 02461 |
| Applicant's Phone(s), Fax and Email 617-719-5500 anyholt 48420 grail com |
| Applicant is one of the following: (please check appropriate box) *Written Permission of the owner is |
| Owner Prospective Buyer* Other* |
| |
| Owner's Name and Address Amy Hot 113 Castle Rd Truco MA 02666 |
| Representative's Name and Address John Ferro 60 Whidah Way WellFleet MAO2 |
| Representative's Phone(s), Fax and Email 508-255-8292 Pine Knoll 123 Cg mall. Co |
| • The applicant is <i>advised</i> to consult with the Building Commissioner, Planning Department, Conservation Department, Health Department, and/or Historic Commission, as applicable, prior to submitting this application. |
| Signature(s) John Ferro Amy Holt |
| Applicant(s)/Representative Printed Name(s) Owner(s) Printed Name(s) or written permission |
| The Any Note |
| Applicant(s)/Representative Signature Owner(s) Signature or written permission |

To the members of the Zoning Board of Appeals,

My name is John Ferro and I work at Pine Knoll. I am the general contractor for the residential project at 113 Castle Road. The reason I'm coming to this board is due to a height restriction conflict. You'll find all the required information in your packets and a summary of the details below.

The asbuilt data showed top of ridge height of the new construction is 1'-6" over Truro's maximum height tolerance. The first two things that happened next were; 1) Find the cause of the problem and

2) Find a solution.

I reviewed the paperwork and emails and found that I had identified this issue late in the design phase and lowered the pitch of the roof in the drawings, but the truss designer never received the update. So the taller trusses got installed and caused the house to be built 1'-6" over maximum height.

I contacted the truss designer to work out a solution. I had them design an engineered truss adjustment that can be applied to the existing structure to lower the ridge height by 1'-9". This put's the new ridge height 3" below tolerance. The adjustment requires working in the attic and on the roof. Once I had the solution, I went to schedule the labor. The framers were unavailable, so I got on their waiting list.

Since I knew I would not be able to make the correction before the summer, I went back to the building department to see if there was a safe way to get the homeowners in the house temporarily for the summer. We came to a conclusion that included a temporary CO and a notarized letter of intent.

The letter of intent includes;

- A. Complete final inspections for electrical, plumbing, gas, and smoke/fire before homeowners move in for the summer.
- B. Allow homeowners to live in the house for the summer (July, August, and September).
- C. Final energy inspection will be delayed until the ridge is lowered.
- D. Final CO will be delayed until the ridge is lowered and the insulation/energy is completed.
- E. Pine Knoll will make the truss adjustment at no extra cost to the homeowner, between end of summer and end of year.
- F. The homeowners and the general contractor signed and notarized a document laying out these guidelines.

Once I had the letter of intent, the building commissioner said that this should be brought in front of the Zoning Board of Appeals before moving forward.

So I'm not asking the board for a height variance. I am asking for the board's approval to proceed within the guidelines of the notarized letter of intent that I created with the building commissioner. If the only way to do that is to be granted a variance, then I am asking you to grant the variance and I will still abide by the notarized letter of intent.

Thank you for your consideration, John Ferro Pine Knoll



TOWN OF TRURO

Assessors Office Certified Abutters List Request Form



| | | DATE: 5 | 122/24 |
|--------------------------------------|--------------------------|--------------------------------|-------------------------------|
| NAME OF APPLICANT: Am | y Holt | DATE | |
| NAME OF AGENT (if any): | na Ferro | (Pine Knol | 1) |
| MAILING ADDRESS: PO Bo | x 1347 N. | Easthan MA | 02651 |
| CONTACT: HOME/CELL 508 | -255-8292 | EMAIL Prokroll 12 | 23@guay.com |
| PROPERTY LOCATION: 13 | Castle Rd | | |
| | (street | address) | |
| PROPERTY IDENTIFICATION NUM | MBER: MAP 46 | PARCEL 398 | EXTif condominium) |
| ABUTTERS LIST NEEDED FOR: | | FEE: \$ | 15.00 per checked item |
| (please check <u>all</u> applicable) | (Fee must accompar | ny the application unless othe | er arrangements are made) |
| Board of Health ⁵ | Planning Board (PB) | Zoning l | Board of Appeals (ZBA) |
| Cape Cod Commission | Special Permit | | Special Permit ¹ |
| Conservation Commission ⁴ | Site Plan ² | X | Variance ¹ |
| Licensing | Preliminary Su | ıbdivision ³ | |
| Type: | Definitive Sub | division ³ | |
| Other | | | (Fee: Inquire with Assessors) |
| | (Please Specify) | | |
| Note: Per M.G.L., process | ing may take up to 10 ca | lendar days. Please plan | accordingly. |
| THIS SECT | TION FOR ASSESSOR | S OFFICE USE ONLY | |
| Date request received by Assessors: | 5/22/24 D | Pate completed: | |
| List completed by: | 1 100 | Pate paid: 5/22/2014 C | ash/Check _/5\ |
| Date request received by Assessors: | 5/22/24 D | Pate completed: | ash/Check 15 |

¹Abutters, owners of land directly opposite on any public or private street or way, and abutters to the abutters within 300 feet of the property line.

²Abutters to the subject property, abutters to the abutters, and owners of properties across the street from the subject property.

³Landowners immediately bordering the proposed subdivision, landowners immediately bordering the immediate abutters, and landowners located across the streets and ways bordering the proposed subdivision. Note: For Definitive Subdivision only, responsibility of applicant to notify abutters and produce evidence as required.

⁴All abutters within 300 feet of parcel, except Beach Point between Knowles Heights Road and Provincetown border, in which case it is all abutters within 100 feet. Note: Responsibility of applicant to notify abutters and produce evidence as required.

⁵Abutters sharing any boundary or corner in any direction – including land across a street, river or stream. <u>Note</u>: Responsibility of applicant to notify abutters and produce evidence as required.



TRURO ASSESSORS OFFICE

PO Box 2012 Truro, MA 02666 Telephone: (508) 214-0921

Fax: (508) 349-5506

Date: May 22, 2024

To: John Ferro, Pine Knoll

From: Assessors Department

Certified Abutters List: 113 Castle Road (Map 46 Parcel 398)

Variance

Attached is a combined list of abutters for 113 Castle Road (Map 46 Parcel 398)

The current owners are Paul and Amy Holt.

The names and addresses of the abutters are as of May 17, 2024 according to the most recent documents received from the Barnstable County Registry of Deeds.

Certified by:

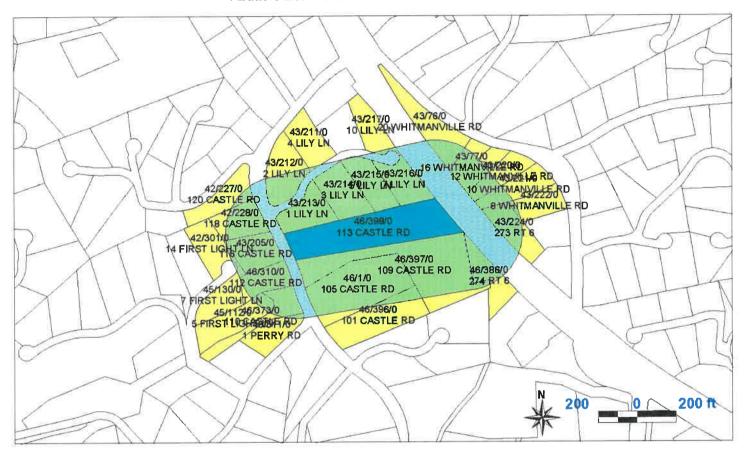
Laura Geiges

Assistant Assessor / Data Collector

113 Castle Rd Map 46 Parcel 398 Variance

TOWN OF TRURO, MA BOARD OF ASSESSORS P.O. BOX 2012, TRURO MA 02666

Abutters List Within 300 feet of Parcel 46/398/0



| Key | Parcel ID | Owner | Location | Mailing Street | Mailing City | ST | ZipCd/Country |
|------|------------|--|--------------------|------------------------------------|-----------------|----|---------------|
| 1809 | 42-227-0-R | WALTER & PAMELA THIEVON REV TR TRS: WALTER & PAMELA THIEVON | 120 CASTLE RD | 367 HIGH ST | STIRLING | NJ | 07980 |
| 1810 | 42-228-0-R | POULIN ETHAN & RODERICK ANDREA | 118 CASTLE RD | PO BOX 263 | NO TRURO | MA | 02652-0263 |
| 5684 | 42-301-0-R | ROBERTS FAMILY REV TRUST TRS:ROBERTS RICHARD E & ANNE B | 14 FIRST LIGHT LN | PO BOX 1280 | TRURO | MA | 02666 |
| 1951 | 43-76-0-R | DEERING LEE SCOTT | 20 WHITMANVILLE RD | PO BOX 299 | NO TRURO | MA | 02652 |
| 1952 | 43-77-0-R | PREGENT SARAH & GASPIE RITA & DENNIS & PAUL | 16 WHITMANVILLE RD | c/o DENNIS GASPIE 19 ENSIGN AVE | MECHANICVILLE | NY | 12118 |
| 1953 | 43-78-0-R | CORREA CHRISTOPHER M & KATIE A | 14 WHITMANVILLE RD | PO BOX 880 | NO TRURO | MA | 02652 |
| 5682 | 43-205-0-R | GUBAR MARTIN D & RHODES REBECCA E | 116 CASTLE RD | 54 JORDAN ROAD | BROOKLINE | MA | 02446 |
| 5898 | 43-211-0-R | WOODBURY SCOTT C & JEANNE A | 4 LILY LN | 3 CENTER WOODS NORTH | SAGINAW | MI | 48638 |
| 5899 | 43-212-0-R | WOODBURY SCOTT C & JEANNE A | 2 LILY LN | 3 CENTER WOODS NORTH | SAGINAW | МІ | 48638 |
| 6018 | 43-213-0-R | NADEAU THOMAS J C/O CARPENTER CLYDE TIMOTHY & | 1 LILY LN | 1803 WHITE OAK HOLLOW NE | ATLANTA | GA | 30324 |
| 6019 | 43-214-0-R | BRATSKEIR STANLEY & BRATSKEIR ANNE REZNIKOFF | 3 LILY LN | PO BOX 1276 | TRURO | MA | 02666 |
| 6020 | 43-215-0-R | COSTA PETER M & MELISSA A | 5 LILY LN | 75 WASHINGTON ST, SUITE 100 | PEMBROKE | MA | 02359 |
| 6021 | 43-216-0-R | JCM REAL ESTATE LLC RES AGT: SHARON S MANDLI | 7 LILY LN | 12770 TOUCHSTONE PLACE | WEST PALM BEACH | FL | 33418 |
| 6022 | 43-217-0-R | DUARTE COLLEEN S & PILLOW MICHAEL J | 10 LILY LN | PO BOX 682 | NO TRURO | MA | 02652 |
| 6665 | 43-220-0-R | MOTTA BRANDON F &MAKER SARAH A | 12 WHITMANVILLE RD | PO BOX 930 | NO TRURO | MA | 02652-0930 |

16 5/22/24

| Key | Parcel ID | Owner | Location | Mailing Street | Mailing City | ST | ZipCd/Country |
|------|------------|---|--------------------|--------------------|------------------|----|---------------|
| 6666 | 43-221-0-R | SOUZA JEFFREY J & SOUZA RACHEL C | 10 WHITMANVILLE RD | PO BOX 690 | NO TRURO | MA | 02652 |
| 6667 | 43-222-0-R | A F HULTIN & CO INC | 8 WHITMANVILLE RD | PO BOX 504 | NO TRURO | MA | 02652-0504 |
| 6669 | 43-224-0-R | KANE CARTER T & AN R | 273 RT 6 | PO BOX 456 | TRURÖ | MA | 02666-0456 |
| 2215 | 45-112-0-R | BIKOFSKY LYNN ANNE | 5 FIRST LIGHT LN | PO BOX 364 | TRURO | MA | 02666 |
| 5687 | 45-130-0-R | BRAUER SHELLEY B & HEY JEAN LD | 7 FIRST LIGHT LN | 4 CALVIN RD | JAMAICA PLAIN | MA | 02130-3415 |
| 2229 | 46-1-0-R | MADSEN PETER E | 105 CASTLE RD | PO BOX 1309 | TRURO | MA | 02666 |
| 2519 | 46-310-0-R | TATERKA JAMES ANDREW & TONI M | 112 CASTLE RD | 827 HAIN DR | LAFAYETTE HILL | PA | 19444 |
| 2520 | 46-311-0-R | LEFORT PAUL F & EILEEN M TRS TRS: LEFORT PAUL F & EILEEN M | 1 PERRY RD | 860 SADDLEWOOD DR | GLEN ELYN | IL | 60137 |
| 5681 | 46-373-0-R | WOLFSON JANE | 110 CASTLE RD | 10 BATES ST | CAMBRIDGE | MA | 02140 |
| 6664 | 46-386-0-R | GARVAN STEVEN J & CAREN L | 274 RT 6 | PO BOX 398 | N.TRURO | MA | 02652 |
| 7414 | 46-396-0-R | D MAYERS & L BOWMAN REV TRUST TRS:DAMON MAYERS & LYNN BOWMAN | 101 CASTLE RD | PO BOX 322 | TRURO | MA | 02666 |
| 7416 | 46-397-0-R | HAGHIGHI MOHAMAD T | 109 CASTLE RD | 19 STONEY BROOK RD | HOPKINTON | MA | 01748 |
| 7417 | 46-398-0-R | HOLT PAUL & AMY | 113 CASTLE RD | 75 ANDREW ST | NEWTON HIGHLANDS | MA | 02461 |

42-228-0-R

43-77-0-R

42-301-0-R

43-78-0-R

43-212-0-R

WALTER & PAMELA THIEVON REV TR TRS: WALTER & PAMELA THIEVON 367 HIGH ST STIRLING, NJ 07980

POULIN ETHAN & RODERICK ANDREA PO BOX 263 NO TRURO, MA 02652-0263 ROBERTS FAMILY REV TRUST TRS:ROBERTS RICHARD E & ANNE B PO BOX 1280 TRURO, MA 02666

43-76-0-R

PREGENT SARAH &
GASPIE RITA & DENNIS & PAUL
c/o DENNIS GASPIE

CORREA CHRISTOPHER M & KATIE A
PO BOX 880

DEERING LEE SCOTT PO BOX 299 NO TRURO, MA 02652

19 ENSIGN AVE
MECHANICVILLE, NY 12118

NO TRURO, MA 02652

GUBAR MARTIN D & RHODES REBECCA E 54 JORDAN ROAD BROOKLINE, MA 02446

WOODBURY SCOTT C & JEANNE A 3 CENTER WOODS NORTH SAGINAW, MI 48638 WOODBURY SCOTT C & JEANNE A 3 CENTER WOODS NORTH SAGINAW, MI 48638

43-213-0-R

43-205-0-R

43-214-0-R

43-211-0-R

43-215-0-R

NADEAU THOMAS J C/O CARPENTER CLYDE TIMOTHY & 1803 WHITE OAK HOLLOW NE ATLANTA, GA 30324 BRATSKEIR STANLEY & BRATSKEIR ANNE REZNIKOFF PO BOX 1276 TRURO, MA 02666

COSTA PETER M & MELISSA A 75 WASHINGTON ST, SUITE 100 PEMBROKE, MA 02359

43-216-0-R

43-217-0-R

43-220-0-R

JCM REAL ESTATE LLC RES AGT: SHARON S MANDLI 12770 TOUCHSTONE PLACE WEST PALM BEACH, FL 33418 DUARTE COLLEEN S & PILLOW MICHAEL J PO BOX 682 NO TRURO, MA 02652

MOTTA BRANDON F &MAKER SARAH A PO BOX 930 NO TRURO, MA 02652-0930

43-221-0-R

43-222-0-R

43-224-0-R

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45-112-0-R

45-130-0-R

46-1-0-R

BIKOFSKY LYNN ANNE PO BOX 364 TRURO, MA 02666 BRAUER SHELLEY B & HEY JEAN LD 4 CALVIN RD JAMAICA PLAIN, MA 02130-3415 MADSEN PETER E PO BOX 1309 TRURO, MA 02666

46-310-0-R

46-311-0-R

46-373-0-R

TATERKA JAMES ANDREW & TONI M 827 HAIN DR LAFAYETTE HILL, PA 19444 LEFORT PAUL F & EILEEN M TRS TRS: LEFORT PAUL F & EILEEN M 860 SADDLEWOOD DR GLEN ELYN, IL 60137

WOLFSON JANE 10 BATES ST CAMBRIDGE, MA 02140

46-386-0-R

46-396-0-R

46-397-0-R

GARVAN STEVEN J & CAREN L PO BOX 398 N.TRURO, MA 02652 D MAYERS & L BOWMAN REV TRUST TRS:DAMON MAYERS & LYNN BOWMAN PO BOX 322 TRURO, MA 02666

HAGHIGHI MOHAMAD T 19 STONEY BROOK RD HOPKINTON, MA 01748

46-398-0-R

HOLT PAUL & AMY 75 ANDREW ST NEWTON HIGHLANDS, MA 02461

LG 5/22/24

HOLTPROJECT

105 CASTLE RD.

TRURO, MA

SUMMARY OF CONSTRUCTION REQUIREMENTS

STANDARD FRAMING CONNECTION REQUIREMENTS:

FOLLOW REQUIREMENTS OF TABLE 2 FROM WECM MANUAL.

FLOOR CONSTRUCTION REQUIREMENTS:

FIRST TWO JOIST BAYS OF THE FLOOR FRAMING FROM EACH GABLE END TO BE BLOCKED WITH TJI BLOCKING OR 2x LUMBER 4-ft ON CENTER FOR THE LENGTH OF THE JOIST. SHEATHING TO BE NAILED IN ACCORDANCE WITH TABLE 2 (8d NAILS, 6" SPACING AT THE EDGES AND 12" SPACING IN THE FIELD).

EXTERIOR WALL REQUIREMENTS

ALL EXTERIOR WALL STUDS TO BE 2x6 AT 16" ON CENTER. THE DOUBLE TOP PLATES ON THE EXTERIOR WALLS TO HAVE A MAXIMUM SPLICE LENGTH OF 6 FEET AND SPLICES TO BE NAILED WITH 20-16 & NAILS IN ACCORDANCE WITH TABLE 6 IN THE WFCM 110/B BOOKLET.

ROOF FRAMING REQUIREMENTS:

RAFTER CONNECTION TO THE TOP PLATE REQUIRES SIMPSON H2.5A
HURRICANE CLIPS WITH 2X BLOCKING BETWEEN JOIST BAYS TOE NAILED TO
THE RAFTER AND TOP PLATE WITH 7-IOd NAILS PER BAY. IF BLOCKING IS
NOT DESIRED, SIMPSON H-IOA OR H-I4A HURRICANE CLIPS CAN BE
SUBSTITUTED AND INSTALLED ON EVERY RAFTER WITHOUT BLOCKING. ALL
CLIPS TO BE INSTALL IN ACCORDANCE WITH SIMPSON REQUIREMENTS.

COLLAR TIES ARE REQUIRED IN THE UPPER THIRD OF THE ROOF RAFTERS AND ARE TO BE NAILED WITH (5) IOU NAILS PER SIDE OR USE SIMPSON LSTA 18 STRAPS FROM RAFTER TO RAFTER OVER THE RIDGE BOARD.

ROOF SHEATHING TO BE NAILED USING 8d OR EQUIVALENT NAILS 6" ON CENTER AT THE EDGES, 6" ON CENTER IN THE FIELD. THE FIRST TWO BAYS BETWEEN RAFTERS ARE REQUIRED TO BE BLOCKED 4 FEET ON CENTER AT ALL GABLE ENDS PER THE WFCM.

LIMITATIONS AND CONTRACTOR RESPONSIBILITIES

THE CONTRACTOR MUST REFER TO THE TABLES AND FIGURES WITHIN THE WFCM IIO MPH EXPOSURE B BOOKLET FOR ILLUSTRATIONS AND REQUIREMENTS DISCUSSED WITHIN THIS SUMMARY. ALL CONNECTIONS AND NAILING MUST MEET THE REQUIREMENTS HEREIN AND AS ILLUSTRATED IN THE BOOKLET IN ORDER TO BE IN COMPLIANCE WITH THE BUILDING CODE. THE CONTRACTOR IS RESPONSIBLE TO ENSURE ALL CONNECTIONS, NAILING, AND ANCHOR BOLTS ARE VISIBLE TO THE INSPECTOR AT THE TIME OF THE FRAMING INSPECTION/ FOUNDATION INSPECTION. THE CONTRACTOR MUST REFERENCE THE SIMPSON STRONG TIE C-2014 CATALOG FOR ALL STRAP, HANGAR, AND TIE INSTALLATION REQUIREMENTS AND LIMITATIONS. THIS DOCUMENT AND THE ATTACHMENTS AS WELL AS A COPY OF THE WFCM BOOKLET MUST ACCOMPANY ALL SETS OF PLANS SUBMITTED TO THE BUILDING DEPARTMENT AND ISSUED TO THE

CONTRACTOR/ SUBCONTRACTORS UNLESS THE PLANS ARE UPDATED WITH NOTES AND DETAILS THAT REFLECT THE REQUIREMENTS STATED IN THIS DOCUMENT AND ATTACHMENTS.

AWC Guide to Wood Construction in High Wind Areas: 110 mph Wind Zone Massachusetts Checklist for Compliance (780 CMR 5301.2.1.1)

| WIND SPEED (3-SEC. GUST) | IIO MPH | X |
|---|--|---------------------------------------|
| WIND EXPOSURE CATEGORY | В | X |
| 1.2 APPLICABILITY | | |
| | PRES ≤ 2 STORES | <u> </u> |
| ROOF PITCH MEAN ROOF HEIGHT | <u>4: 2</u> ≤ 2: 2 22 ft ≤33' | |
| BUILDING WIDTH, W | 48 ft ≤80' | |
| BUILDING LENGTH, L | 50 ft ≤80' | X |
| BUILDING ASPECT RATIO (L/W) | <u>-50</u> :05:1 ≤3:1 | X |
| NOMINAL HEIGHT OF TALLEST OPENING | <u>6'8"</u> ≤6'8" | X |
| 1.3 FRAMING CONNECTIONS GENERAL COMPLIANCE WITH FRAMING CONNE | EC110NS | X |
| 2.1 FOUNDATION | | |
| FOUNDATION WALLS MEET REQ. OF 780 CMF | R 5404.1 - CONCRETE | |
| 2.2 ANCHORAGE TO FOUNDATIONI,3 | | |
| 5/8" ANCHOR BOLTS IMBEDDED OR 5/8" F MECHANICAL ANCHORS AS AN ALTERNATIVE IN | | |
| BOLT SPACING — GENERAL | <u>54</u> in. o.c. | X |
| BOLT SPACING FROM END/JOINT OF PLATE | 9 in, ≤ 6 " - $ 2$ " | X |
| BOLT EMBEDMENT - CONCRETE | _7_ in, ≥ 7" | <u> </u> |
| PLATE WASHER (FIG 5) | ≥ 3" X 3" X ¼" | <u> </u> |
| 3.1 FLOORS | | \ |
| FLOOR FRAMING MEMBER SPANS CHECKED | | <u> </u> |
| MAXIMUM FLOOR OPENING DIMENSION FULL HEIGHT WALL STUDS AT FLOOR OPENING | <u>10</u> ft ≤ 12-ft | X |
| LESS THAN 2' FROM EXTERIOR WALL | 1) | Χ |
| MAX. FLOOR JOIST SETBACKS SUPPORTING | | |
| LOAD BEARING OR SHEAR WALLS | N/A ft $\leq d$ | N/A |
| MAX. CANTILEVERED JOISTS SUPPORTING | | · · · · · · · · · · · · · · · · · · · |
| LOAD BEARING OR SHEAR WALLS | N/A ft $\leq d$ | N/A |
| FLOOR BRACING AT ENDWALLS | | X |
| FLOOR SHEATHING TYPE | | |
| FLOOR SHEATHING THICKNESS | <u>3/4</u> in. | |
| FLOOR SHEATHING FASTENING 8 d NAILS AT 6 in. ED | PGE / 12 in. FIELD | X |
| 4.1 WALLS | | |
| WALL HEIGHT | | |
| LOADBEARING WALLS | <u>9</u> ft ≤ 10' | <u>X</u> |
| NON-LOADBEARING WALLS | <u>9</u> ft ≤ 20" | <u>X</u> |
| WALL STUD SPACING | $\underline{16} \text{ in.} \leq 24 \text{"o.c}$ | X |
| WALL STORY OFFSETS | N/A ft $\leq d$ | <u> N/ A</u> |
| 4.2 EXTERIOR WALLS3 | | |
| WOOD STUDS | - 8 ft 0 : | V |
| LOADBEARING WALLS 2 x <u>6</u> NON-BEARING WALLS 2 x 6 | - <u>8</u> ft <u>9</u> in. - <u>8</u> ft <u>9</u> in. | X |
| GABLE END WALL BRACING | | |
| FULL HEIGHT ENDWALL STUDS | | Χ |
| | 100 % ≥ 0.9W | X |
| I X 3 CEILING FURRING STRIPS @ 16" S | | <u> </u> |
| BLOCKING @ 4 ft. SPACING IN END JO | OIST/TRUSS BAYS | X |
| DOUBLE TOP PLATE | | |
| SPLICE LENGTH | <u>6</u> ft | <u> </u> |
| SPLICE CONNECTION (# 16D COMMO | ON NAILS) 20 | X |

| ompliance (780 CMR | 5301.2.1.1) | |
|--|--|-----------------|
| LOADBEARING WALL CONNECTIONS | | |
| LATERAL (# 16d COMMON | NALS)2 | X |
| NON-LOADBEARING WALL CONNECT | 10N5 | |
| LATERAL (# 16d COMMON | NAILS)2 | <u> </u> |
| LOAD BEARING WALL OPENINGS | | V |
| HEADER SPANS | <u> 0 </u> | <u>X</u> |
| SILL PLATE SPANS | <u> 0</u> ft <u>0</u> in,≤ -f | <u>X</u> |
| FULL HEIGHT STUDS NON-LOAD BEARING WALL OPENING | <u>4</u> | X |
| HEADER SPANS | 1) ft 0 in,≤ 2-ft | X |
| SILL PLATE SPANS | $\frac{1}{ } \text{ ft } 0 \text{ in. } \leq 2\text{-ft} $ | |
| FULL HEIGHT STUDS (NO. OF | - | X |
| EXTERIOR WALL SHEATHING TO RESIS | | DUSLY |
| MINIMUM BUILDING DIMENSIO | 2N (W) | |
| HEIGHT OF TALLEST | OPENING <u>6'8"</u> ≤ 6'8" | <u> </u> |
| SHEATHING TYPE | <u>WSP</u> | <u> </u> |
| EDGE NAIL SPACING | | <u> </u> |
| FIELD NAIL SPACING | | <u> </u> |
| SHEAR CONNECTION | | <u>X</u> |
| PERCENT FULL-HEIGH | | <u>X</u> |
| MAXIMUM BUILDING DIMENSI | PENINGS > 6'8" | |
| HEIGHT OF TALLEST | | Y |
| SHEATHING TYPE | W5P | <u> </u> |
| EDGE NAIL SPACING | | X |
| FIELD NAIL SPACING | | X |
| SHEAR CONNECTION | 1(#16d/ft) <u>3</u> | X |
| PERCENT FULL-HEIGH | H SHEATHING 30 | <u> </u> |
| -5% FOR O | PENINGS > 6'8" | X |
| WALL CLADDING | | |
| RATED FOR WIND SPEED? | MECLENI CHE AMININI I CINCEM | X |
| APA PORTAL WALLS AND / OR WIND | DESIGN SHEARWALLS USED | <u>YES</u> |
| 5.1 ROOFS | | |
| ROOF FRAMING MEMBER SPANS CHE | CKED? | X |
| ROOF OVERHANG I ft. | ≤ SMALLER OF 2-ft OR L/3 | X |
| TRUSS OR RAFTER CONNECTIONS AT | LOAD BEARING WALLS | |
| PROPRIETARY CONNECTORS | | |
| UPLIFT | U= 370 plf | <u> </u> |
| LATERAL | L= 176 plf | <u> </u> |
| SHEAR | S= 77 plf | <u>X</u> |
| RIDGE STRAPS (IF COLLAR TIES NOT | ' | <u>X</u> |
| GABLE RAKE OLITLOOKER I_ ft : 1RUSS OR RAFTER CONNECTIONS AT | | X |
| PROPRIETARY CONNECTORS | NON LUPULINING WILL) | |
| UPLIFT | | N/A |
| LATERAL (#16d CON | MMON NAILS) | N/A |
| ROOF SHEATHING TYPE | WSP | X |
| ROOF SHEATHING THICKNESS | 7/16 in.≥7/1 <mark>6"</mark> WSP | X |
| ROOF SHEATHING FASTENING | | X |
| NOTES: | | |
| I. THIS CHECKLIST SHALL BE MET IN ITS ENTIRE | | |
| 780 CMR 5301.2.1.1 ITEM 1. IF THE CHECKLIS | | |
| METAL STRAPS AND HOLD DOWNS ARE NOT I | REQUIRED PER THE WFCM 110 MPI | H GUIDE: |
| a. STEEL STRAPS PER FIGURE 5 | | |
| b. 20 GAGE STRAPS PER FIGURE II | | |
| c. UPLIFT STRAPS PER FIGURE 14 d. ALL STRAPS PER FIGURE 17 | | |
| e. CORNER STUD HOLD DOWNS PER F | FIGURE IRA AND FIGURE IRB | |
| 2. THE BOTTOM SILL PLATE IN EXTERIOR WAL | | OMINAL. |
| 1HICKNESS PRESSURE TREATED #2-GRADE. | 1 II VV VV I I IVIII VIIVIUIVI Z II VI I VV | ✓ / V III N/ 1/ |
| THE CHIEF CHIEF THE PROPERTY OF THE PROPERTY O | | 40LLC (4.0) |

3. SEE CHECKLIST SHEARWALL CONSTRUCTION DETAIL FOR SHEARWALL CONSTRUCTION

THIS REVIEW WAS COMPLETED ON PLANS SUBMITTED BY PINE KNOLL BUILDERS AND WAS BASED ON THE FLOOR PLANS AND ELEVATIONS PROVIDED. ANY CHANGES TO THESE PLANS OR FIELD CHANGES MADE MAY RENDER THE REQUIREMENTS OUTLINED IN THIS DOCUMENT NULL AND VOID AND COULD RESULT IN NON-COMPLIANCE WITH THE REQUIREMENTS OF THE WIND DESIGN.

SHEARWALL PANEL NAILING SCHEDULE

6/12

¹⁵/₃₂" PLYWOOD NAILED WITH 8d COMMON OR GALVANIZED BOX NAILS AT 6"O.C. AT THE EDGES AND 12" O.C. IN THE FIELD.



 $^{15}\!\!/_{32}$ " PLYWOOD NAILED WITH 8d COMMON OR GALVANIZED BOX NAILS AT 4"O.C. AT THE EDGES AND 12" O.C. IN THE FIELD.



 $^{15}\!\!/_{32}$ " PLYWOOD NAILED WITH 8d COMMON OR GALVANIZED BOX NAILS AT 3"O.C. AT THE EDGES AND 12" O.C. IN THE FIELD.

NOTE: FOR PLYWOOD SHEAR WALLS LISTED ABOVE, 8d COMMON OR GALVANIZED BOX NAILS = $(0.131 \times 21/2^{11})$. GUN NAILS MATCHING THE NAIL DIAMETER AND LENGTH MAY BE USED AS A SUBSTITUTE.

NOTE: ALL PLYWOOD TO BE RUN VERTICAL FROM SILL PLATE TO AT LEAST 2" INTO THE SECOND FLOOR BOX ON TWO STORY BUILDINGS OR TO THE DOUBLE TOP PLATE IN SINGLE STORY BUILDINGS. USE 2 ROWS OF NAILS SPACED 3" ON CENTER STAGGERED AT THE TOP AND BOTTOM OF EACH PLYWOOD SHEET PER FIGURE 4 IN THE CHECKLIST.

SOLE PLATE CONNECTION SCHEDULE

CONNECTION TO FLOOR RIM BOARD

WALL TYP

WALL TYPE SOLE PLATE CONNECTION TO RIM BOARD

6/12

(3) - 16d COMMON NAILS PER 16"

4/12

(3) - 16d COMMON NAILS PER 16"



(4) - 16d COMMON NAILS PER 16"

CONNECTION TO CONCRETE FOUNDATION

FOUNDATION SILL PLATE CONNECTION TO CONCRETE

8" DIA, ANCHOR BOLTS AT 54" O.C.,

 $\frac{\text{NOTE}}{\text{NOTE}}: \text{ANCHOR BOLTS REFERENCED ABOVE TO BE} \\ \frac{5}{8}" \text{ DIAMETER A307 STEEL} \\ \text{ANCHOR BOLTS WITH 3" x 3" x } \\ \frac{1}{4}" \text{ PLATE WASHERS WITH 7" MINIMUM EMBEDMENT INTO CONCRETE.} \\ \\$

SHEARWALL CONSTRUCTION

I - ALL SHEARWALLS TO HAVE DOUBLE TOP PLATES AND DOUBLE 2X STUDS AT EACH END OF THE WALL.

2 - FACE NAIL DOUBLE TOP PLATES W/ 16d NAILS AT 16" O.C.

3 - NAILING OF SHEATHING TO BE CONTINUED ABOVE AND BELOW ALL OPENINGS IN SHEARWALL.

4 - ATTACH DOUBLE 2X STUDS AND BUILT-UP CORNER STUDS AT SHEARWALL ENDS WITH
(2) 16d NAILS AT 6" O.C. FOR ATTIC/SECOND FLOOR SHEARWALLS AND (2) 16d
NAILS AT 4" O.C. STAGGERED FOR FIRST FLOOR SHEARWALLS.

KING AND JACK STUD REQUIREMENTS

XK,XJ # OF KING AND JACK STUDS AT OPENINGS. USE 2K, IJ IF NOT NOTED OTHERWISE

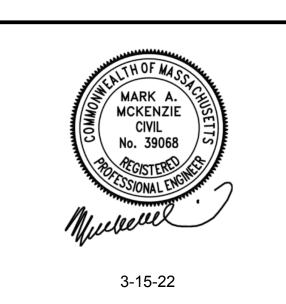
CHECKLIST COVER SHEET PROJECT: HOLT PROJECT

PROJECT ADDRESS:

105 CASTLE RD. TRURO, MA



P.O. BOX 1879 44 UNDERPASS RD UNIT 2 BREWSTER, MA 02631 (774) 353-2144



| JOB#: 22-051 | SHEET |
|-----------------|-------|
| DATE:03-11-2022 | CS1.0 |
| SCALE: NONE | |

CHECKLIST SHEARWALL CONSTRUCTION

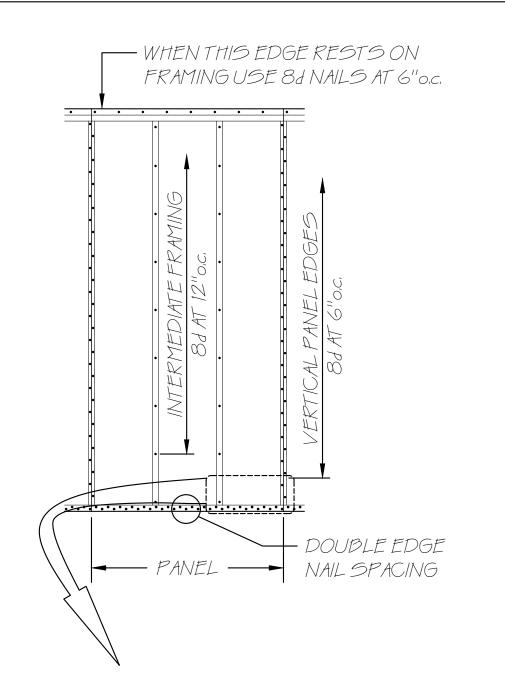
I. FROM TABLES IO AND II WFCM MANUAL IIO MPH EXP. B AND LOCATION OF WALL SHEATHING AND BUILDING ASPECT RATIO, DETERMINE PERCENT FULL-HEIGHT SHEATHING AND NAIL SPACING REQUIREMENTS

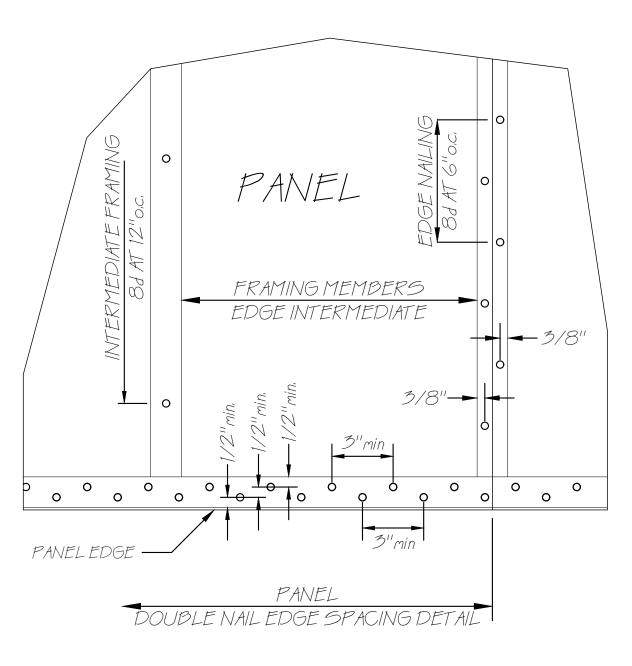
2. WOOD STRUCTURAL PANELS SHALL BE MINIMUM THICKNESS OF 7/16" AND BE INSTALLED AS FOLLOWS:

- a. PANELS SHALL BE INSTALLED WITH STRENGTH AXIS PARALLEL TO STUDS.
- 6. ALL HORIZONTAL JOINTS SHALL OCCUR OVER AND BE NAILED TO FRAMING.
- c. ON SINGLE STORY CONSTRUCTION, PANELS SHALL BE ATTACHED TO BOTTOM PLATES AND TOP MEMBER OF THE DOUBLE TOP PLATE.
- d. ON TWO STORY CONSTRUCTION, UPPER PANELS SHALL BE ATTACHED TO THE TOP MEMBER OF THE UPPER DOUBLE TOP PLATE AND TO BAND JOIST AT BOTTOM OF PANEL. UPPER ATTACHMENT OF LOWER PANEL SHALL BE MADE TO BAND JOIST AND LOWER ATTACHMENT MADE TO LOWEST PLATE AT FIRST FLOOR FRAMING.

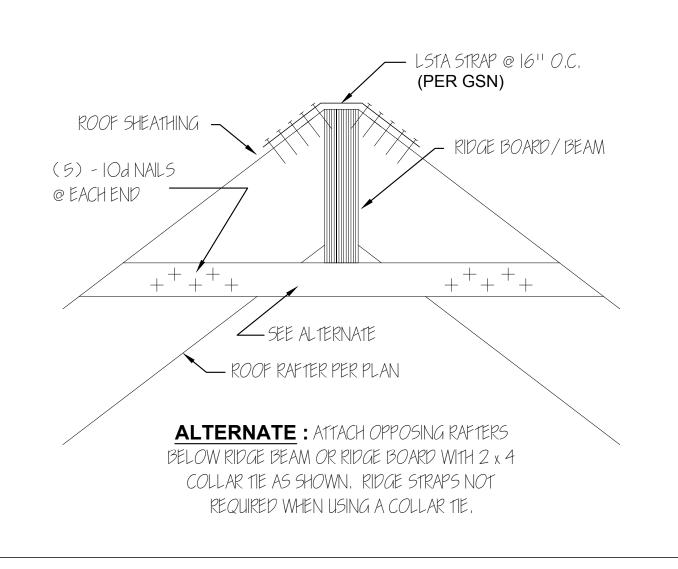
e. HORIZONTAL NAIL SPACING AT DOUBLE TOP PLATES, BAND JOISTS, AND GIRDERS SHALL BE A DOUBLE ROW OF 8D STAGGERED AT 3 INCHES ON CENTER PER FIGURES BELOW: VERTICAL AND HORIZONTAL NAILING FOR PANEL ATTACHMENT

VERTICAL AND HORIZONTAL NAILING FOR PANEL ATTACHMENT

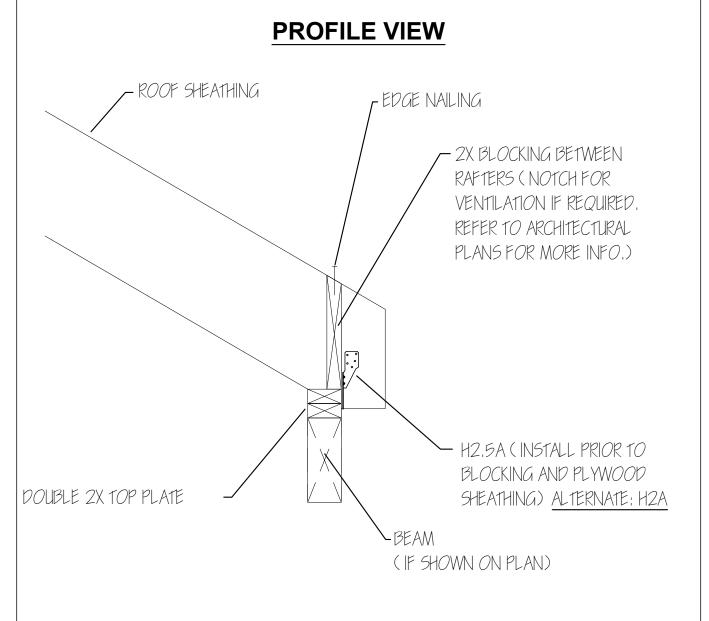




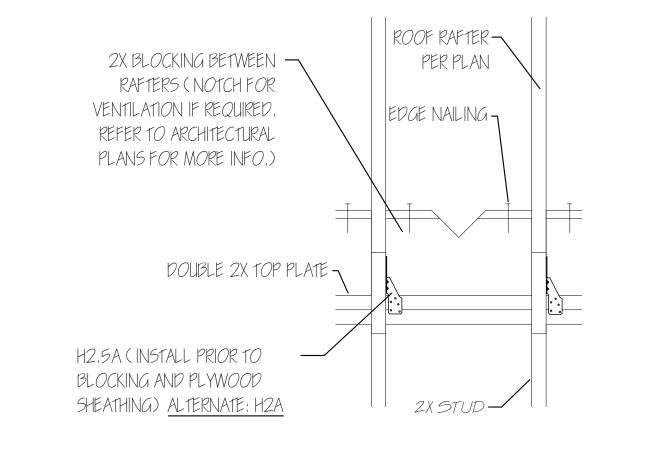
STRUCTURAL RIDGE BEAM



RAFTER TO TOP PLATE



ELEVATION VIEW



SHEARWALL HOLDDOWN SCHEDULE

SECOND FLOOR AND INTERIOR HOLDDOWNS

- (1) CS 16 COIL STRAP W/ (26) 8d (Q.131 x 21/2" LONG) NAILS WITH STRAP APPLIED DIRECTLY TO 2X FRAMING MEMBERS, PROVIDE HALF OF THE NUMBER OF NAILS SPECIFIED AT EACH END OF STRAP. CUT SMALL SLOT IN FLOOR SHEATHING AND ATTACH STRAP TO LVL BEAM OR LVL BLOCKING IN BETWEEN TJI FLOOR JOISTS IN PLOOR FRAMING BELOW, CONNECT BLOCKING TO TJI JOIST WEBS WITH HUS 412 FACE MOUNT HANGER. PROVIDE BACKER BLOCKING IN TJI JOIST WEB PER MANUFACTURER'S SPECIFICATIONS.
- (2) CS 16 COIL STRAP W/ (26) 8d (0.131 x 2½" LONG) NAILS WITH STRAP APPLIED DIRECTLY TO 2X FRAMING MEMBERS, PROVIDE HALF OF THE NUMBER OF NAILS SPECIFIED AT EACH END OF STRAP. CUT SMALL SLOT IN FLOOR SHEATHING AND ATTACH STRAP TO LYLBEAM OR LYLBLOCKING IN BETWEEN TJI FLOOR JOISTS IN FLOOR FRAMING BELOW. CONNECT BLOCKING TO TJI JOIST WEBS WITH HUS 412 FACE MOUNT HANGER, PROVIDE BACKER BLOCKING IN TJI JOIST WEB PER MANUFACTURER'S SPECIFICATIONS.

EARWALL GRIDLINE

FOUNDATION HOLDDOWNS

- HDUA-SDS2.5 W/ SSTB2O ANCHOR BOLT PLACED BEFORE POUR. ATTACH TO FOUNDATION W/ APPLICABLE ANCHORMATE. USE CHW COUPLER NUT BETWEEN ANCHOR BOLT AND & THREADED ROD INTO HOLDOWN.
- STHD14 FOUNDATION HOLDOWN STRAPS FOR APA PORTAL WALLS, SEE TT-100F FOR ADDITIONAL CONSTRUCTION DETAILS, ATTACH HOLDOWNS TO FOUNDATION FORMWORK WITH APPROPRIATE ANCHORMATES PRIOR TO POUR.
- HDU14-SD52,5 ATTACHED TO 6x6 DOUGLAS-FIR POST W/ SBIx30 ANCHOR BOLT PLACED BEFORE POUR. ATTACH TO FORM WORK WITH APPLICABLE ANCHORMATE. USE CNW I''
 COURLER NUT BETWEEN ANCHOR BOLT AND I'' THREADED ROD INTO HOLDOWN.

LEGEND

SHEARWALL TYPE

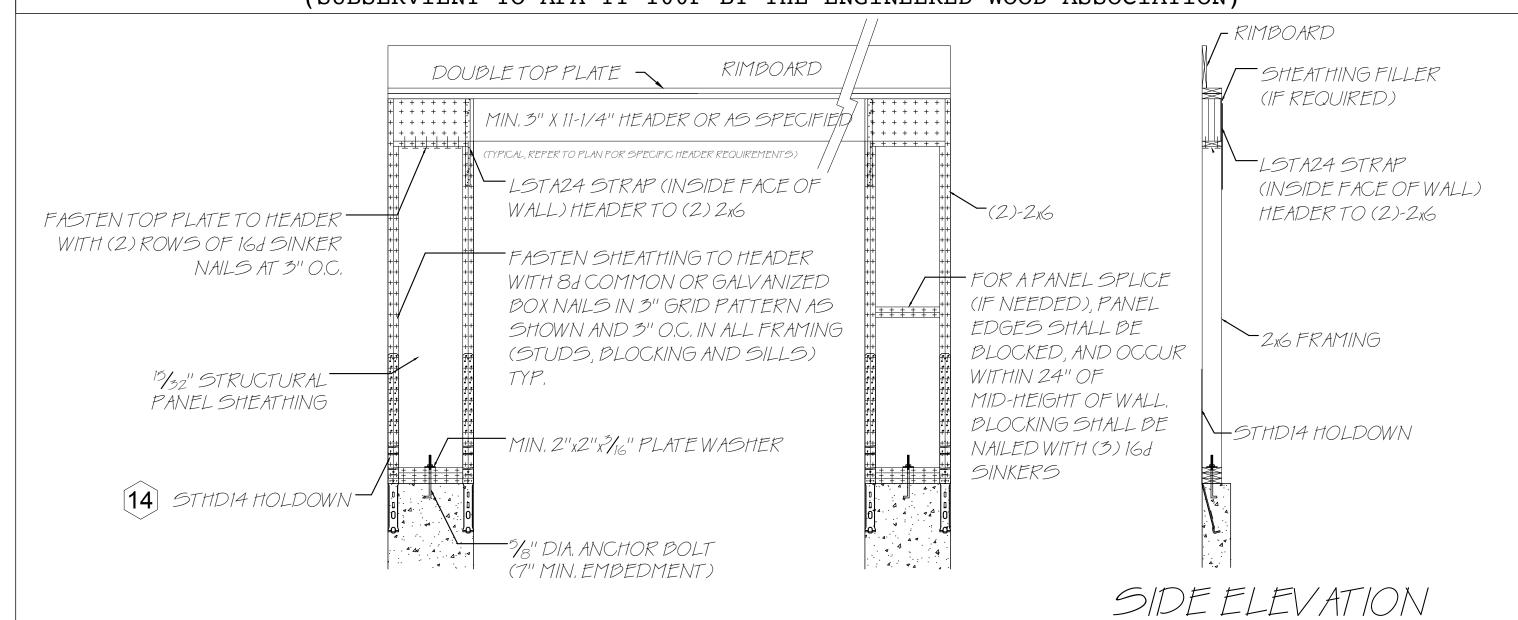
SHEARWALL HOLDDOWN TYPE

OPENING WITH NAILING ACCORDING TO SPECIFIED SHEARWALL TYPE.

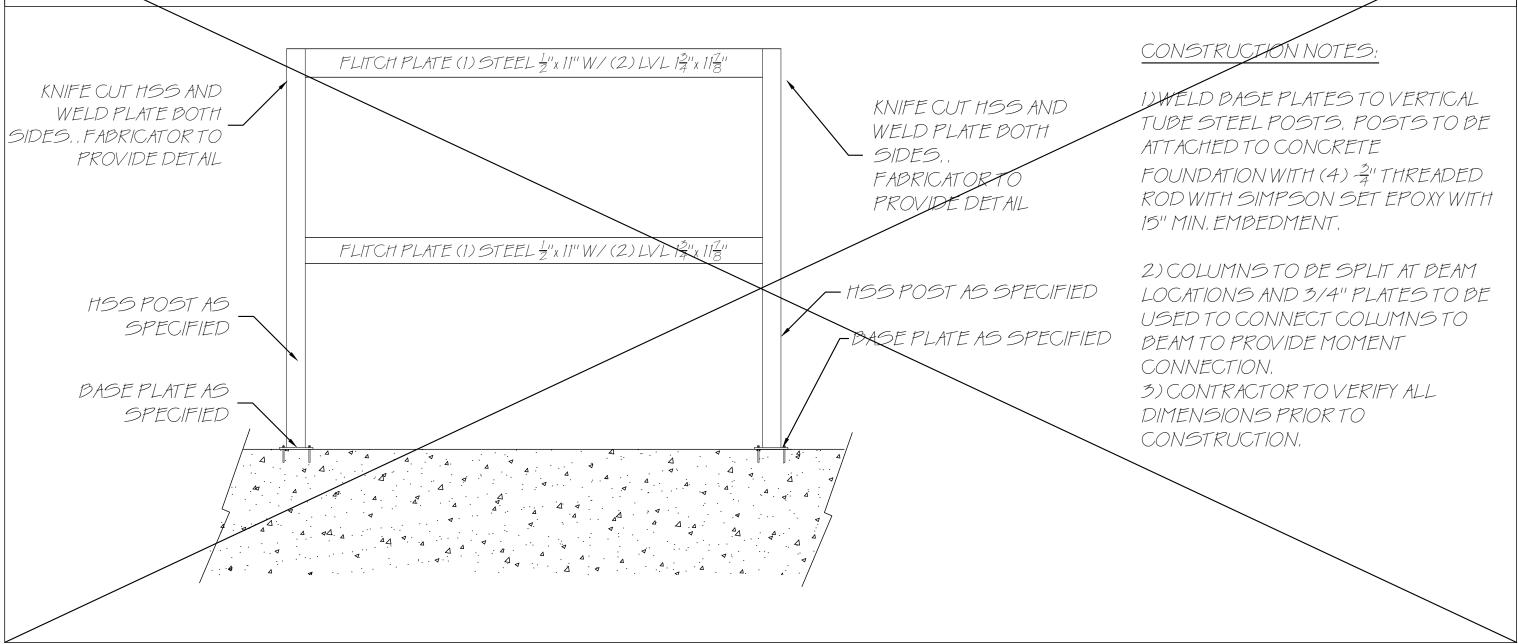
---- SHEARWALL XK, XJ # OF KING AND JACK STUDS AT OPENINGS

APA PORTAL WALL DETAIL (NOT TO SCALE)

(SUBSERVIENT TO APA TT-100F BY THE ENGINEERED WOOD ASSOCIATION)



MOMENT FRAME CONSTRUCTION DETAIL (EXAMPLE ONLY, NOT TO SCALE)



77 7

ADDITIONAL PROJECT:

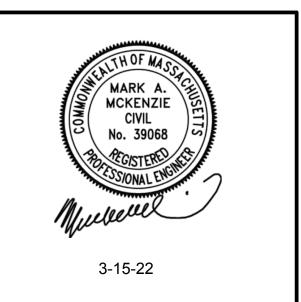
REVISION/ISSUE DATE

PROJECT ADDRESS:

105 CASTLE RD. TRURO, MA



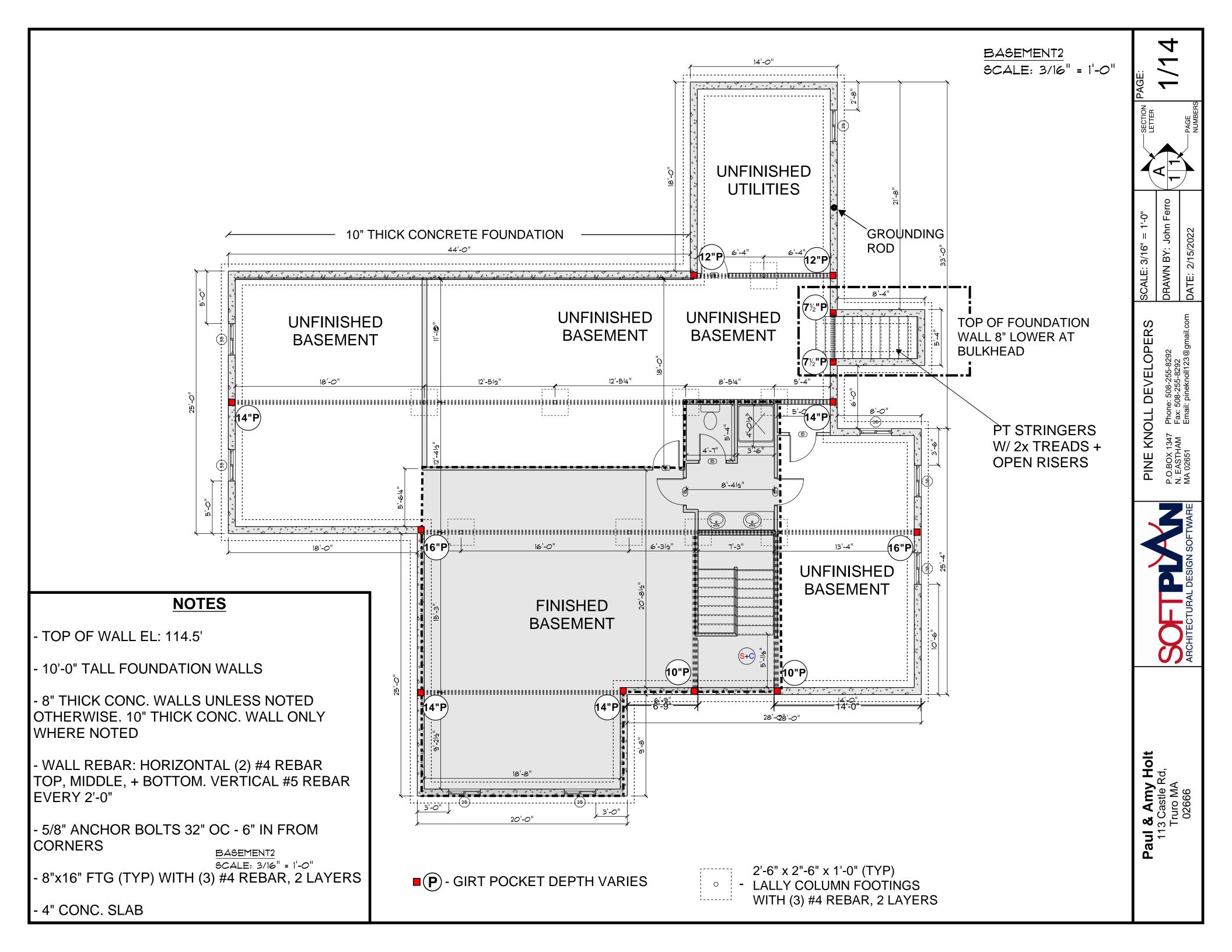
P.O. BOX 1879 44 UNDERPASS RD UNIT 2 BREWSTER, MA 02631 (774) 353-2144

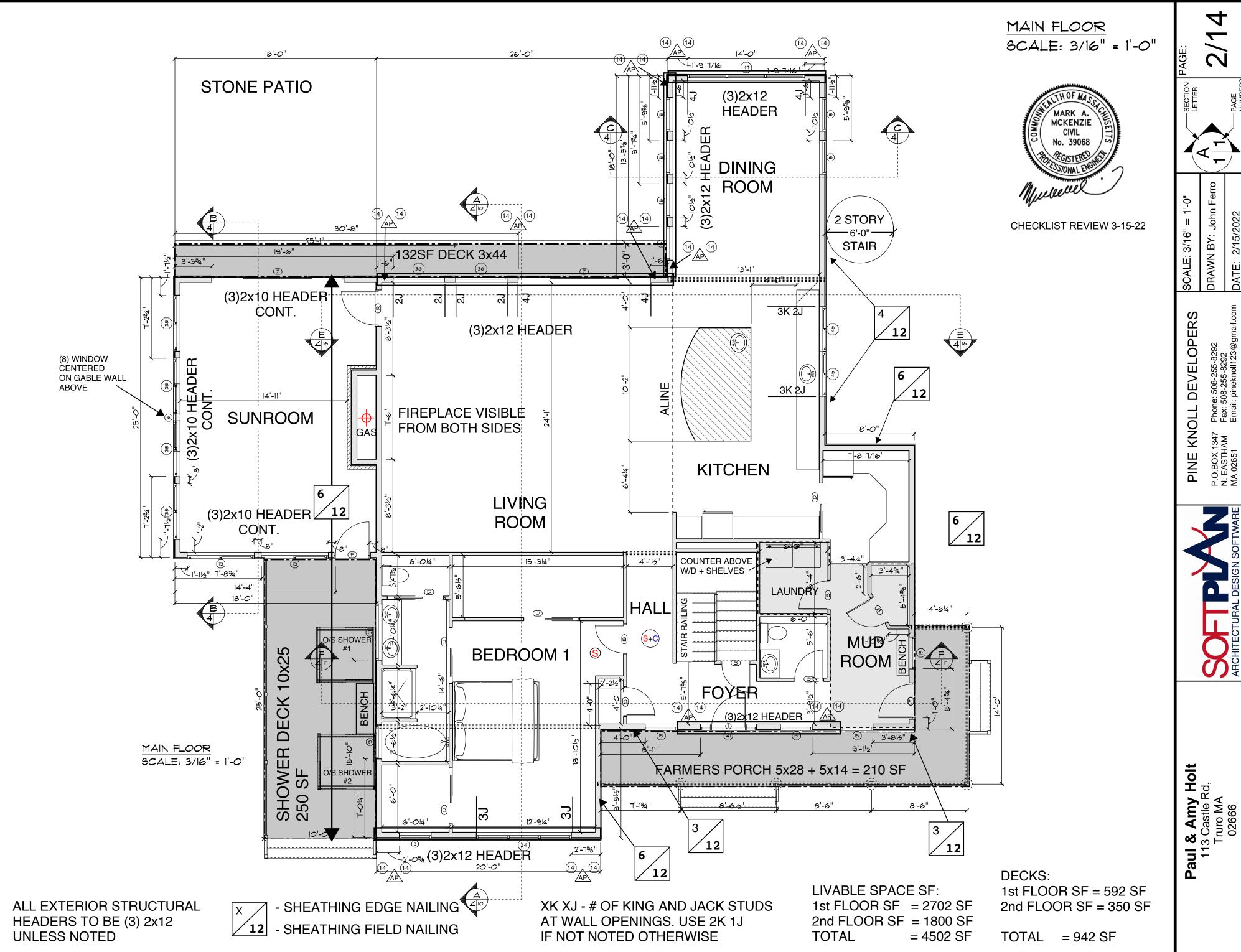


JOB#: 22-051 SHEET

DATE:03-11-2022 CS1.1

SCALE: NONE

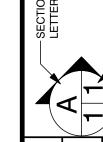




DATE: 2/15/2022

P.O.BOX 1347 N. EASTHAM MA 02651

4



DRAWN BY: John Ferro 2/15/2022

DATE:

P.O.BOX 1347 N. EASTHAM MA 02651

FIRST FLOOR **FINISH SCHEDULE**

SECOND FLOOR FINISH SCHEDULE

4

DRAWN BY: John Ferro SCALE: 3/16" = 1'-0"

DATE: 3

2/15/2022

P.O.BOX 1347 N. EASTHAM MA 02651

Paul & Amy Holt 113 Castle Rd, Truro MA 02666

PLAYROOM + WORKOUT AREA

FLOORS - CARPET + RUBBER FLOOR IN WORKOUT AREA WALLS - SHEETROCK **CEILING - SHEETROCK** LIGHTS - RECESSED

BATH 1

FLOORS - TILE WALLS - SHEETROCK **CEILING - SHEETROCK** LIGHTS - RECESSED + **VANITY LIGHTS**

UNFINISHED BASEMENT

FLOORS - CONCRETE SLAB WALLS - CONCRETE **CEILING - UNFINISHED** LIGHTS - 4' LONG TUBE LIGHTS

SUNROOM:

FLOORS - HARD WOOD WALLS - SHEETROCK FIRE PLACE FINISH - STONE CEILING - CATHEDRAL CEILING FINISH - NATURAL WOOD CEILING FAN - YES LIGHTS - RECESSED

LIVING ROOM:

FLOORS - HARD WOOD WALLS - SHEETROCK FIRE PLACE FINISH - STONE CEILING - SHEETROCK LIGHTS - RECESSED

KITCHEN:

FLOORS - HARD WOOD WALLS - SHEETROCK, BACKSPLASH TILE + SOFFIT ABOVE WALL CABS (SIZE: TBD) CEILING - SHEETROCK LIGHTS - RECESSED + (2) PENDANT LIGHTS

DINING ROOM:

FLOORS - HARD WOOD WALLS - SHEETROCK CEILING - SHEETROCK LIGHTS - HANGING FIXTURE

BATH 2:

FLOORS - TILE WALLS - SHEETROCK + BACKSPLASH TILE CEILING - SHEETROCK | CEILING FAN - YES |LIGHTS - RECESSED + ||LIGHTS - RECESSED **VANITY LIGHTS**

FOYER:

FLOORS - OAK WALLS - SHEETROCK **CEILING - SHEETROCK LIGHTS - RECESSED**

GARAGE:

FLOORS - CONCRETE WALLS - SHEETROCK **CEILING - SHEETROCK** LIGHTS - TBD

BEDROOM 1:

FLOORS - CARPET WALLS - SHEETROCK CEILING - SHEETROCK

1/2 BATH 1 + W/D ROOM:

FLOORS - TILE WALLS - SHEETROCK **CEILING - SHEETROCK** LIGHTS - RECESSED

MUDROOM:

FLOORS - TILE WALLS - SHEETROCK CEILING - SHEETROCK LIGHTS - RECESSED

BEDROOMS 2, 3, 4, & 5

FLOORS - HARD WOOD WALLS - SHEETROCK CEILING - CATHEDRAL, WOOD BEAMS, SHEETROCK CEILING FAN: YES (1/ea) LIGHTS - RECESSED

BATHS 3, 4, & 5

FLOORS - TILE WALLS - SHEETROCK CEILING - SHEETROCK LIGHTS - RECESSED + **VANITY LIGHTS**

DECKS + WIDOWS WALK:

DECK - HEAT RESISTANT (GREY) COMPOSITE DECKING POSTS - PRESSURE TREATED RAILING - WOOD POST W/ CABLE RAILING

SITTING ROOM + HALLWAYS:

FLOORS - HARD WOOD WALLS - SHEETROCK CEILING - CATHEDRAL + NATURAL WOOD CEILING FAN: YES LIGHTS - RECESSED

DATE: 2/15/2022

| aul & Amy Holt | 113 Castle Rd, | Truro MA | 02666 |
|----------------|----------------|----------|-------|
| Ø | | | |

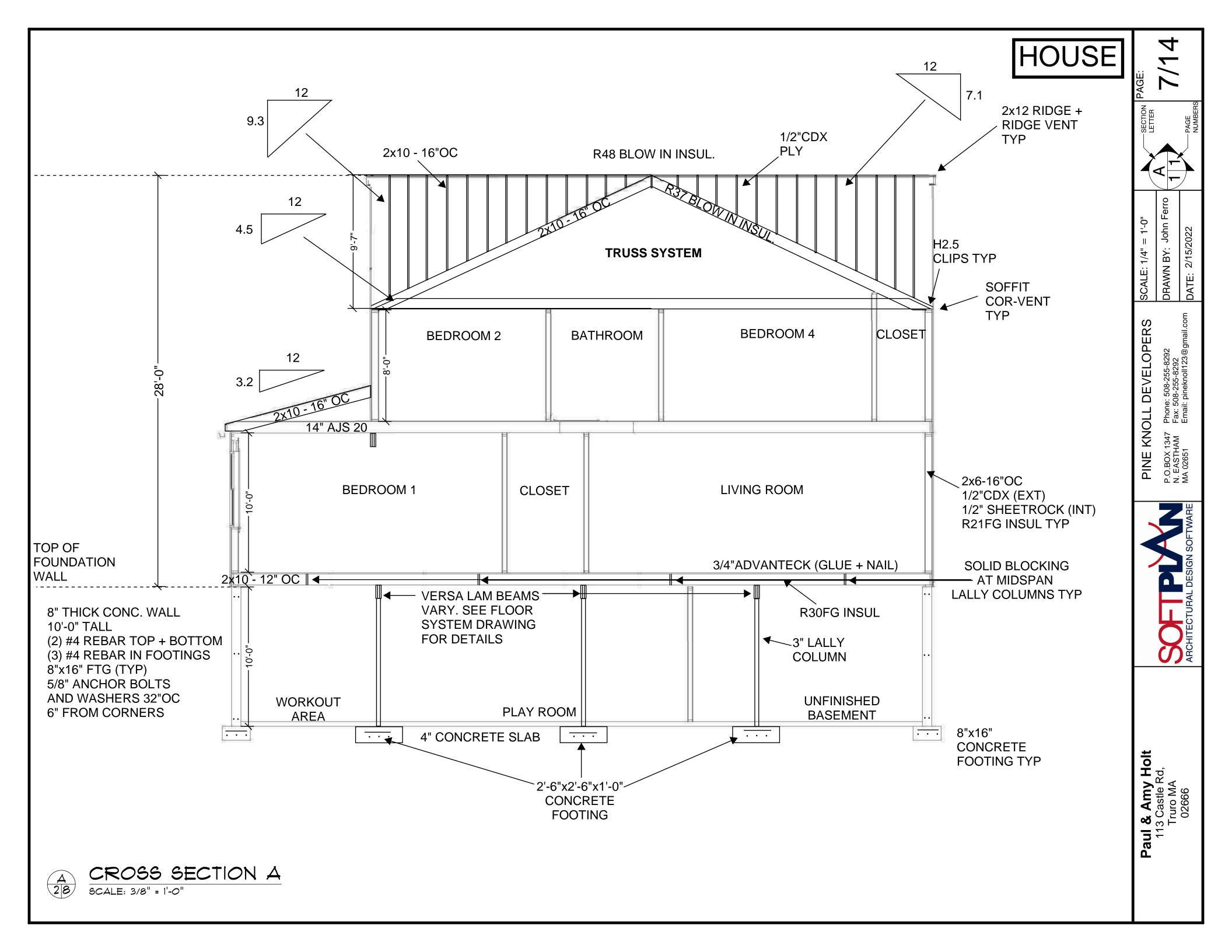
| | | WINDOW SCHEDULE | | | | | | |
|----------------------------|------------|--------------------------|-------|---|-----------------------|-----------------------------|--|--|
| | OPENING ID | PRODUCT CODE | COUNT | LIBRARY NAME | TYPE | R.O. SIZE | | |
| count as 15 - | 3 | TW2852 | 1 | Manufacturer\Andersen\400-Tilt-Wash Double Hung | WINDOW | R.O. 2'-10 1/8" x 5'-4 7/8" | | |
| | 4 | TW2842 | 16 | Manufacturer\Andersen\400-Tilt-Wash Double Hung | WINDOW | R.O. 2'-10 1/8" x 4'-6 7/8" | | |
| | 5 | TW210510 | 6 | Manufacturer\Andersen\400-Tilt-Wash Double Hung | WINDOW | R.O. 3'-0 1/8" x 6'-0 7/8" | | |
| | 6 | TW2842-3 | 2 | Manufacturer\Andersen\400-Tilt-Wash Double Hung | WINDOW | R.O. 8'-6 7/8" x 4'-6 7/8" | | |
| | 8 | CTC3 (Half Round) | 1 | Manufacturer\Andersen\400-Casement_Awning Picture_Transom Windows | WINDOW | R.O. 6'-0 3/8" x 3'-2 3/4" | | |
| | 9 | PTR3010 | 1 | Manufacturer\Andersen\400-Casement_Awning Picture_Transom Windows | WINDOW | R.O. 3'-0 1/2" x 1'-0 1/2" | | |
| | 15 | TW2852 | 5 | Manufacturer\Andersen\400-Tilt-Wash Double Hung | WINDOW | R.O. 2'-10 1/8" x 5'-4 7/8" | | |
| | 19 | G65 + 1'-6" Transom | 2 | Manufacturer\Andersen\400-Gliding Windows + 1'-6" Transom | COMBINED UNIT | R.O. 6'-0" x 6'-6 1/8" | | |
| | 21 | PTR5010 | 1 | Manufacturer\Andersen\400-Casement_Awning Picture_Transom Windows | WINDOW | R.O. 5'-0 3/8" x 1'-0 1/2" | | |
| | 22 | FLX 10'x4' | 1 | Manufacturer\Andersen\400-Flexiframe 10'x4' | WINDOW | R.O. 10'-0 1/2" x 4'-0 5/8" | | |
| | 29 | TW2832 | 1 | Manufacturer\Andersen\400-Tilt-Wash Double Hung | WINDOW | R.O. 2'-10 1/8" x 4'-6 7/8" | | |
| | 34 | TW2852-3 | 1 | Manufacturer\Andersen\400-Tilt-Wash Double Hung | WINDOW | R.O. 8'-6 7/8" x 5'-4 7/8" | | |
| oundation windows — | 35 | 36X24 SLIDER | 8 | Window\Slider | SLIDING WINDOW | R.O. 3' 0" x 2' 0" | | |
| | 36 | DHP56410 + 1'-6" Transom | 2 | Manufacturer\Andersen\400-Tilt-Wash DH Picture_Transom Windows | COMBINED UNIT | R.O. 5'-7 7/8" x 6'-7" | | |
| | 38 | G55 + 1'-6" Transom | 4 | Manufacturer\Andersen\400-Gliding Windows + 1'-6" Transom | COMBINED UNIT | R.O. 5'-0" x 6'-6 1/8" | | |
| front door transom - | 41 | DHT41010-MODIFIED | 1 | Manufacturer\Andersen\400-Tilt-Wash DH Picture_Transom Windows | WINDOW | R.O. 5'-0" x 2'-0" | | |
| | 44 | G42 | 1 | Manufacturer\Andersen\400-Gliding Windows | SLIDING WINDOW | R.O. 4'-0" x 1'-11" | | |
| | 45 | G43 | 2 | Manufacturer\Andersen\400-Gliding Windows | SLIDING WINDOW | R.O. 4'-0" x 3'-0" | | |
| | 50 | VS606 | 1 | Velux Skylight S606 | Skylight | R.O. 44 1/4" x 45 3/4" | | |

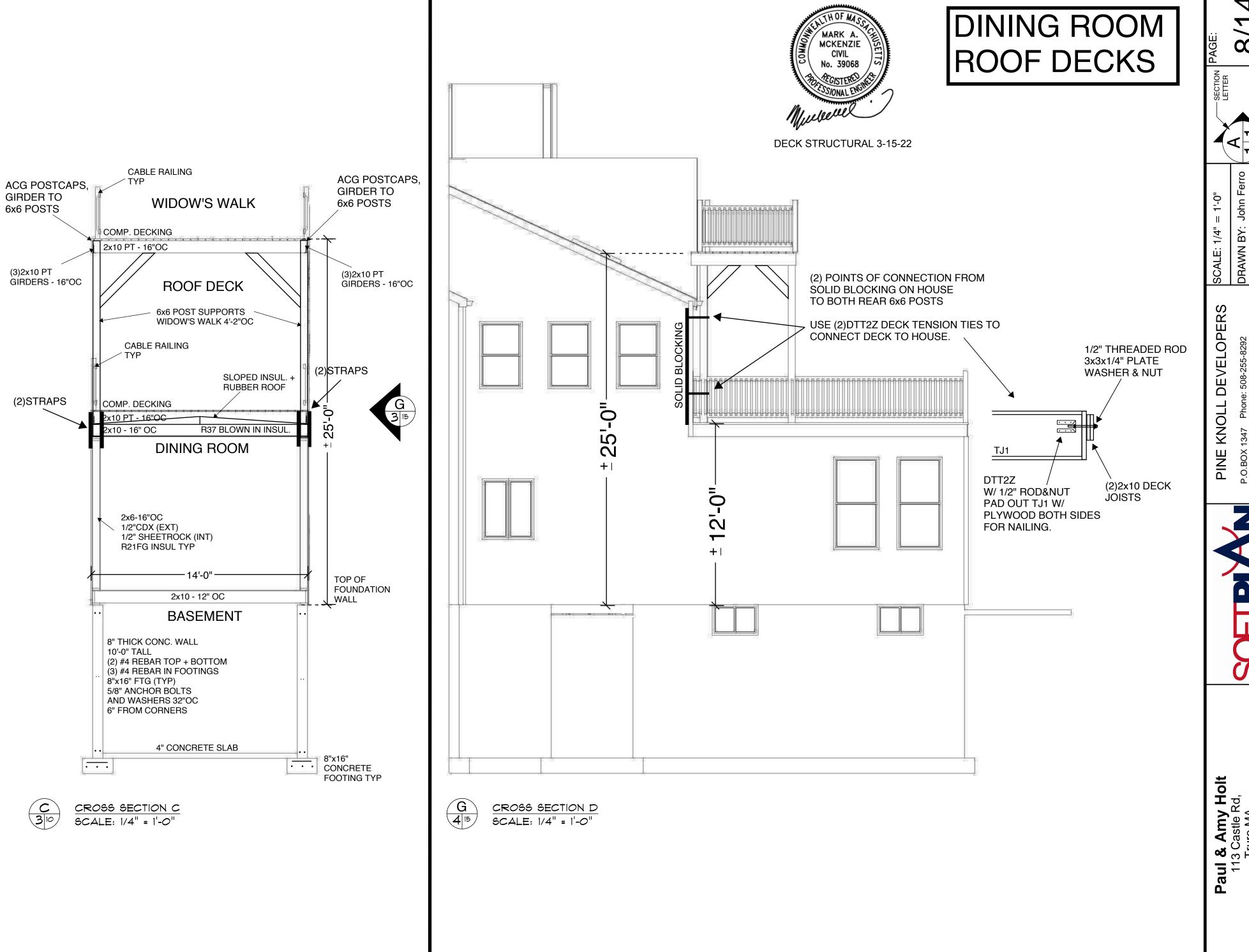
| | DOORS SCHEDULE | | | | | | |
|------------|--|-------|--|---------------|---------------------|--|--|
| OPENING ID | PRODUCT CODE | COUNT | LIBRARY NAME | TYPE | R.O. SIZE | | |
| A | 72X80 SLIDING GLASS 2 | 1 | Exterior Door\Patio | SLIDING DOOR | R.O. 6'-0" x 6'-8" | | |
| В | 30X80 COLONIAL A 1 | 17 | Interior Door\Colonial | DOOR | R.O. 2'-8" x 6'-9" | | |
| D | 30X80 COLONIAL POCKET 1 | 8 | Interior Door\Pocket | POCKET | R.O. 5'-0" x 6'-8" | | |
| E | 36X80 COUNTRY A 1 + 1'-6" Transom | 2 | Exterior Door\Country | COMBINED UNIT | R.O. 3'-3" x 8'-8" | | |
| F | 36X80 COUNTRY A 1 No Grilles Transom MODIFIED | 1 | Exterior Door\Country | COMBINED UNIT | R.O. 3' 3" x 8' 8" | | |
| | 60X80 LH ENTRY - 2 SL-MODIFIED + 1'-6" Transom | 1 | Exterior Door\Entry | COMBINED UNIT | R.O. 5'-0" x 6'-9" | | |
| L | 60X80 BIFOLD COLONIAL 2 | 1 | Interior Door\Bifold | BIFOLD | R.O. 5'-0" x 6'-8" | | |
| M | 54X80 BIFOLD COLONIAL 2 | 1 | Interior Door\Bifold | BIFOLD | R.O. 4'-6" x 6'-8" | | |
| ?? R ?? | 36X80 COUNTRY A 1-No Grilles (2x6 Wall) | 1 | Exterior Door\Country | DOOR | R.O. 3'-3" x 6'-9" | | |
| Т | 30X80 BIFOLD COLONIAL 1 | 2 | Interior Door\Bifold | BIFOLD | R.O. 2'-6" x 6'-8" | | |
| U | 42X80 BIFOLD COLONIAL 2 | 2 | Interior Door\Bifold | BIFOLD | R.O. 3'-6" x 6'-8" | | |
| Z | FWG120611-4 SAAS + 1'-6" Transom | 2 | Manufacturer\Andersen\400-Frenchwood Gliding Patio Doors | COMBINED UNIT | R.O. 11'-9" x 8'-8" | | |



LEFT ELEVATION
SCALE: 1/8" = 1'-0"

RIGHT ELEVATION SCALE: 1/8" = 1'-0"

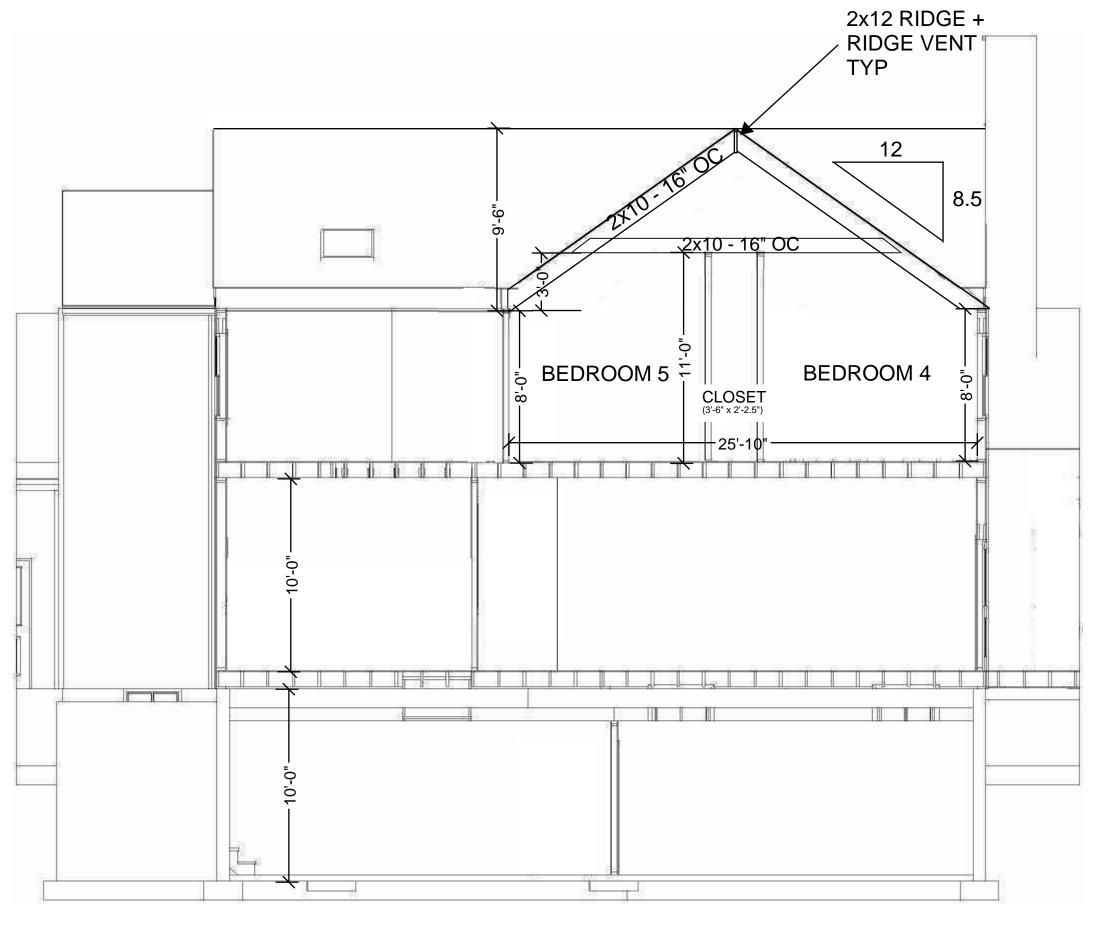




4

DATE: 2/15/2022 DRAWN BY:

P.O.BOX 1347 N. EASTHAM MA 02651



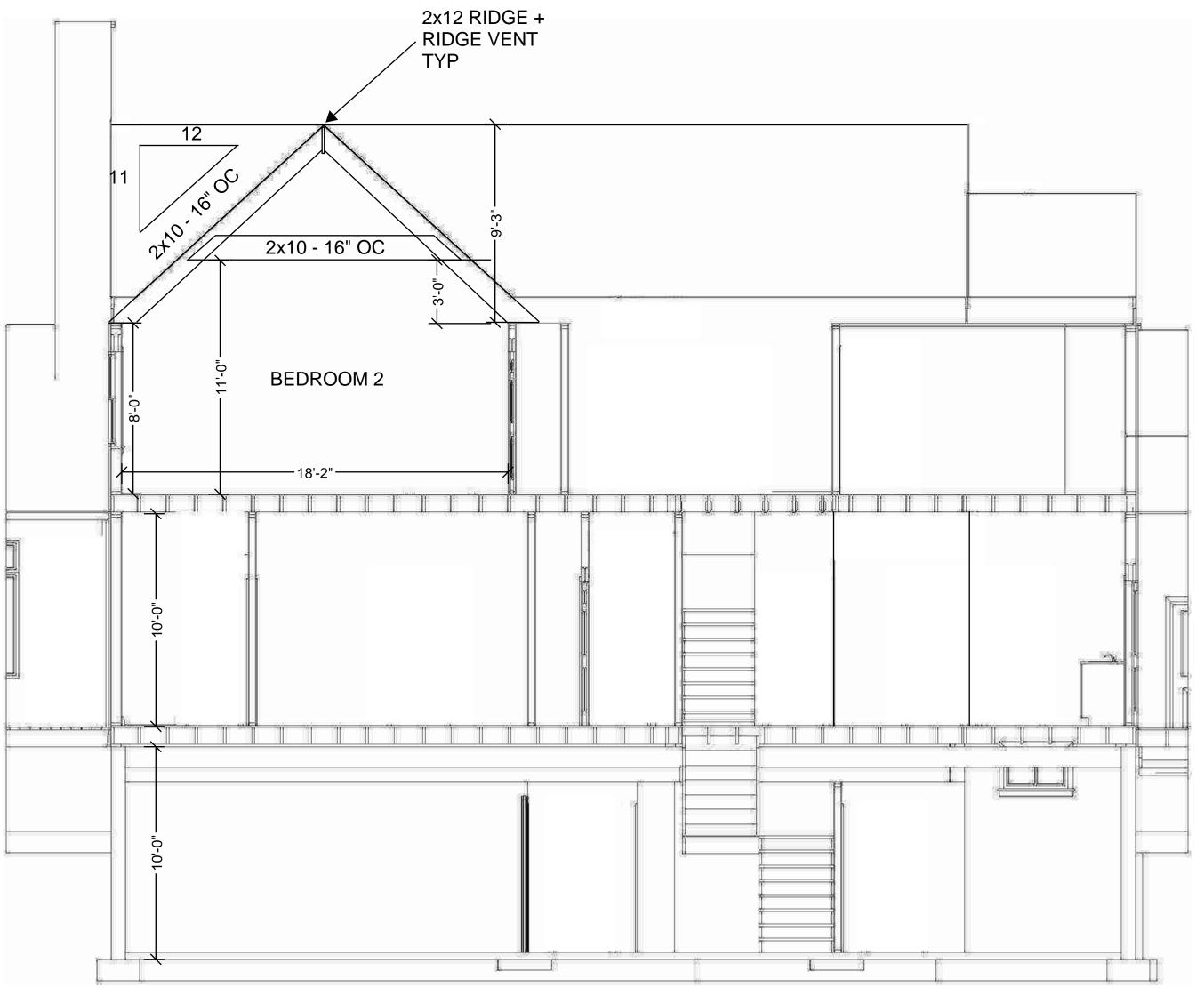
CROSS SECTION E SCALE: 1" = 5'-0"

DRAWN BY: John Ferro

DATE: 2/15/2022

SCALE: 1" = 5'-0"

PINE KNOLL DEVELOPERS



DRAWN BY: John Ferro

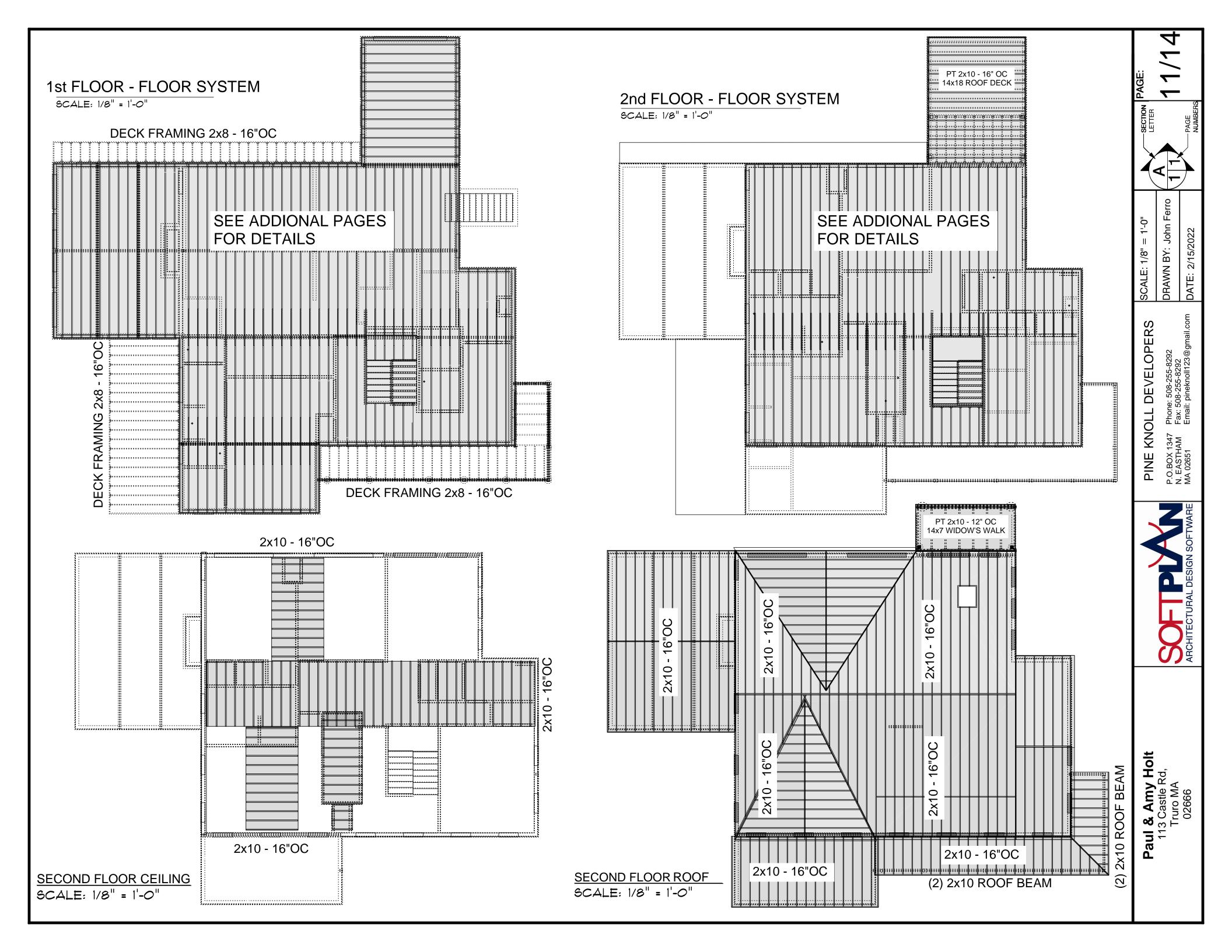
PINE KNOLL DEVELOPERS
P.O.BOX 1347 Phone: 508-255-8292
N. EASTHAM Fax: 508-255-8292
MA 02651 Email: pineknoll123@gmail.com

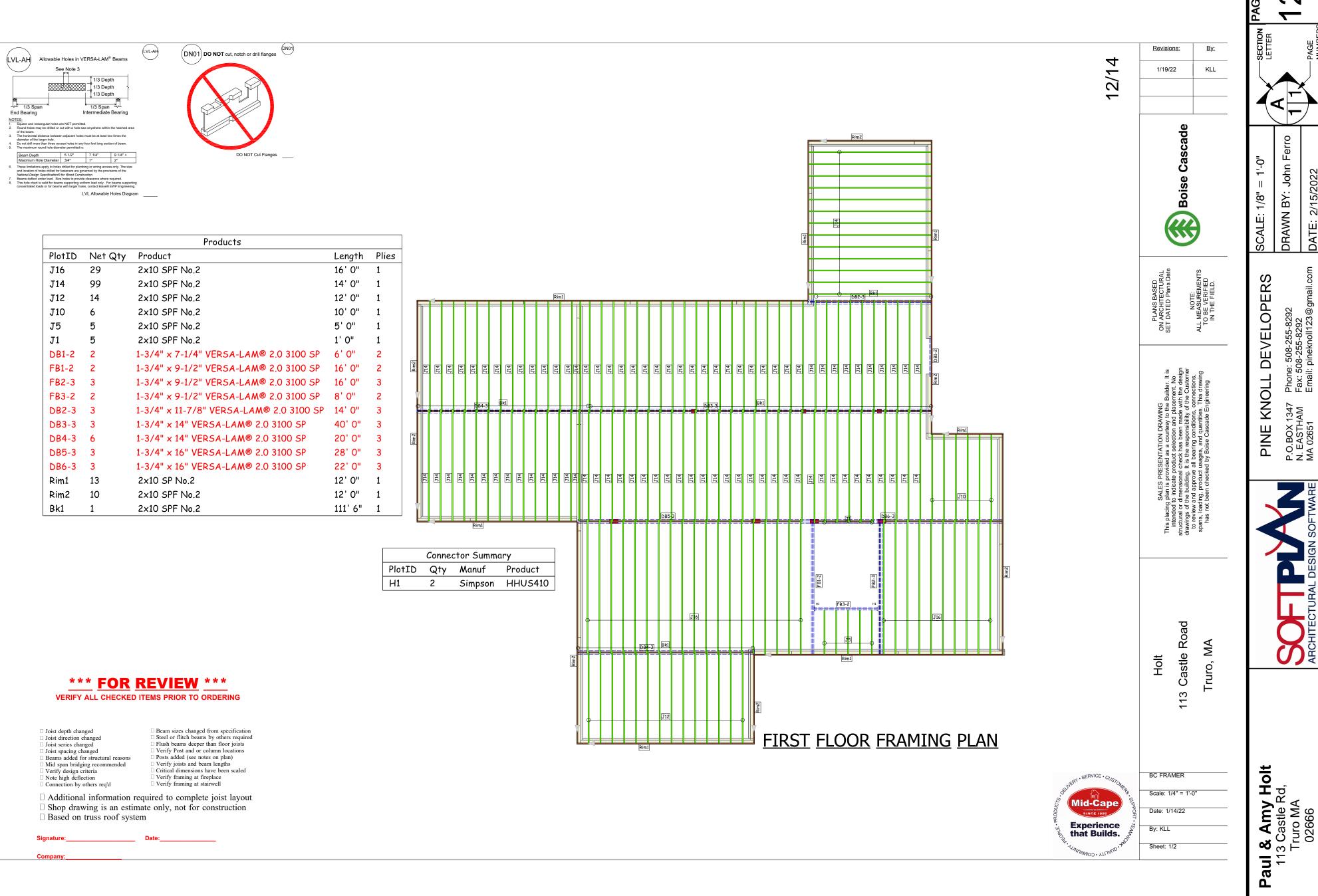
DATE: 2/15/2022

SCALE: 1/4" = 1'-0"

Paul & Amy Holt 113 Castle Rd, Truro MA 02666

CROSS SECTION F SCALE: 1/4" = 1'-0"





PINE KNOLL DEVELOPERS

P.O.BOX 1347 N. EASTHAM MA 02651

DRAWN BY: John Ferro

DATE: 2/15/2022

Revisions:

Boise Cascade

PLANS BASED ON ARCHITECTURAL SET DATED Plans Date

13/14

DRAWN BY: John Ferro SCALE: 1/8" = 1'-0"

DATE: 2/15/2022

SOFT PESIGN SOFTWARE

Paul & Amy Holt 113 Castle Rd, Truro MA 02666

SALES PRESENTATION DRAWING
This placing plan is provided as a courtesy to the Builder. It is
intended to indicate product selection and placement. No
structural or dimensional check has been made with the design
drawings of the building. It is the responsibility of the Customer
to review and approve all bearing conditions, connections,
spans, loading, product usages, and quantities. This drawing
has not been checked by Boise Cascade Engineering

3 Castle Road Truro, MA

BC FRAMER Scale: 1/4" = 1'-0" Date: 1/14/22 By: KLL Sheet: 2/2

Mid-Cape

Experience

that Builds.

SECOND FLOOR FRAMING PLAN 14" AJS-20/AJS-25 @ 16"/12" <u>O.C.</u>

| Products | | | | | | | |
|----------|---------|-------------------------------------|--------|-------|--|--|--|
| PlotID | Net Qty | Product | Length | Plies | | | |
| J40 | 16 | 14" AJS® 20 | 40' 0" | 1 | | | |
| J16 | 13 | 14" AJS® 20 | 16' 0" | 1 | | | |
| J10 | 6 | 14" AJS® 20 | 10' 0" | 1 | | | |
| J6 | 5 | 14" AJS® 20 | 6' 0" | 1 | | | |
| J26 | 18 | 14" <i>A</i> JS® 25 | 26' 0" | 1 | | | |
| FB1-3 | 3 | 1-3/4" x 14" VERSA-LAM® 2.0 3100 SP | 26' 0" | 3 | | | |
| FB2-3 | 3 | 1-3/4" x 14" VERSA-LAM® 2.0 3100 SP | 22' 0" | 3 | | | |
| FB3-2 | 4 | 1-3/4" x 14" VERSA-LAM® 2.0 3100 SP | 16' 0" | 2 | | | |
| FB4-2 | 2 | 1-3/4" x 14" VERSA-LAM® 2.0 3100 SP | 8' 0" | 2 | | | |
| Rim1 | 13 | 1-1/8" x 14" BC RIM BOARD OSB | 12' 0" | 1 | | | |

| Connector Summary | | | | | | |
|-------------------|-----|---------|-------------|--|--|--|
| PlotID | Qty | Manuf | Product | | | |
| H1 | 4 | Simpson | HHUS410 | | | |
| H2 | 24 | Simpson | IUS2.56/14 | | | |
| Н3 | 14 | Simpson | IUS2.56/9.5 | | | |
| H4 | 18 | Simpson | IUS3.56/14 | | | |



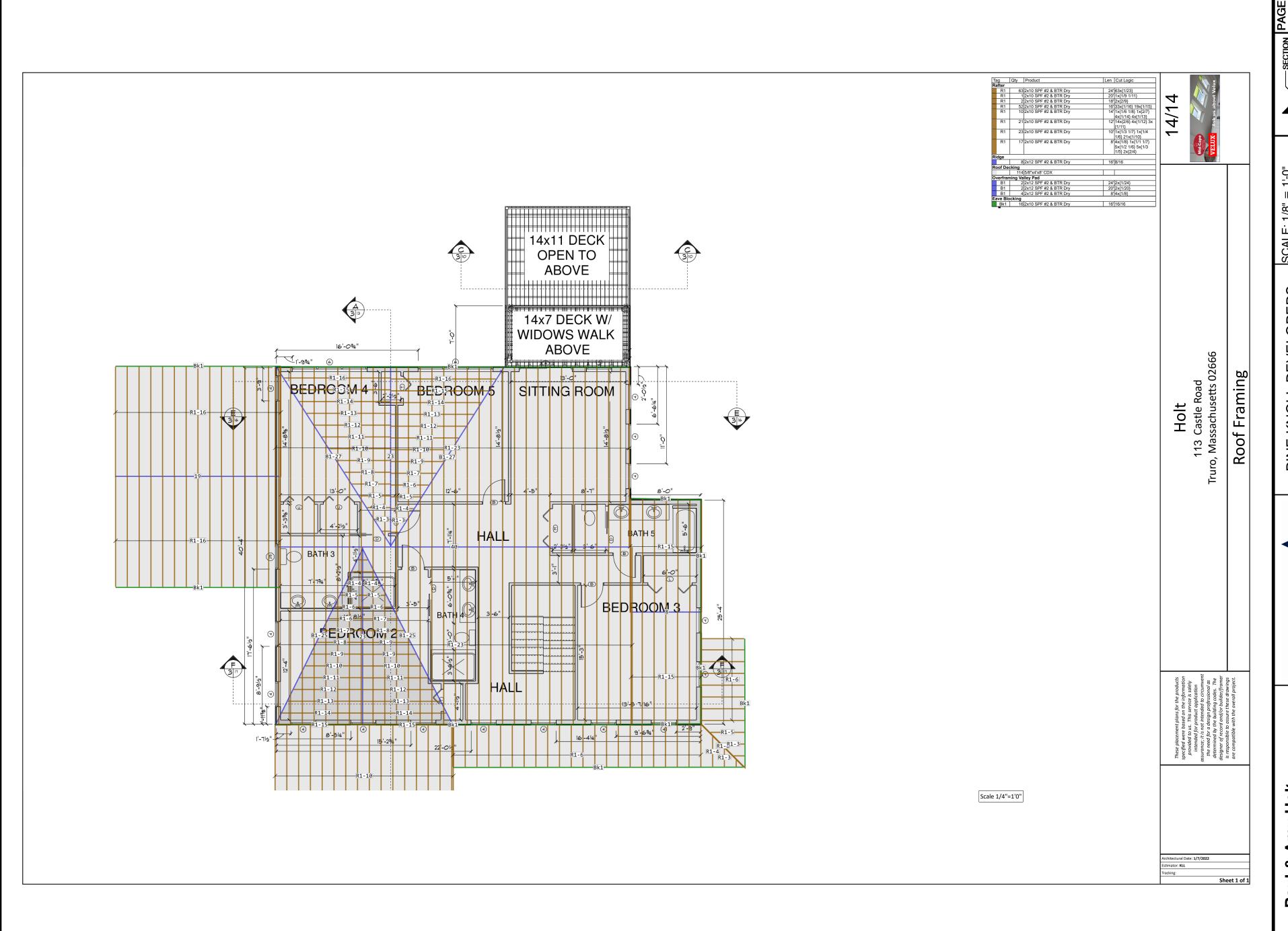
*** **FOR REVIEW** ***

- ☐ Beam sizes changed from specification ☐ Steel or flitch beams by others required ☐ Joist depth changed☐ Joist direction changed☐ ☐ Flush beams deeper than floor joists ☐ Verify Post and or column locations Joist series changed Joist spacing changed
- ☐ Posts added (see notes on plan)
 ☐ Verify joists and beam lengths Beams added for structural reasons ☐ Mid span bridging recommended ☐ Verify design criteria Critical dimensions have been scaled Note high deflection ☐ Verify framing at fireplace ☐ Verify framing at stairwell ☐ Connection by others req'd
- ☐ Additional information required to complete joist layout

☐ Shop drawing is an estimate only, not for construction ☐ Based on truss roof system

PINE KNOLL DEVELOPERS

P.O.BOX 1347 Phone: 508-255-8292 N. EASTHAM Fax: 508-255-8292 MA 02651 Email: pineknoll123@gmail.com



PINE KNOLL DEVELOPERS

P.O.BOX 1347 Phone: 508-255-8292 N. EASTHAM Fax: 508-255-8292 MA 02651 Email: pineknoll123@gmail.com

DATE: 2/15/2022

DRAWN BY: John Ferro SCALE: 1/8" = 1'-0"

14/1 -SECTION LETTER

EXHIBIT 1

TOWN OF TRURO APPLICATION FOR A CURB CUT PERMIT

<u>Note:</u> This permit application must be accompanied by a plan. If this permit is being applied for by someone other than the Owner of the property, the owner's signature must appear at the bottom of the application.

Date: __4/7/22

| To the Board of Selectmen 24 Town Hall Road P. O. Box 2030 Truro, MA 02666 |
|---|
| Re: APPLICATION FOR A CURB CUT |
| Dear Board Members: |
| The applicant(s) hereby make application for a curb cut as follows: |
| Owners Name(s) (Please Print): Paul & Amy Holt |
| Address: 75 Andrew St. Newton, MA 02461 |
| Phone Number: 617-719-5500 |
| Email Address: amyholt4842@gmail.com |
| Curb Cut Street Location: 113 Castle Rd. Truro |
| Affected Town or State road: Castle Rd |
| Truro Assessor's Map Number: 46 Parcel Number: 398 |
| Name of contractor: Pine Knoll Dev. |
| Contractor Phone Number: 508-255-8292 |
| Contractor Email: pineknoll123@gmail.com |
| Reason/explanation: New Construction |
| I/we hereby agree to the terms and conditions as outlined in this policy and attached Exhibits: |
| Applicant's Signature: 4/7/22 |
| Owner's Signature (if different): |



TOWN OF TRURO

P.O. Box 2030, Truro MA 02666
Tel: (508) 349-7004 Fax: (508) 349-5505

POLICY MEMORANDUM #28

Date: Adopted June 6, 2000, revised 9/22/04, 2/28/06, 6/13/06, 10/13/07

Subject: CURB CUT POLICY

1. Introduction

Due to the continuing growth in construction activity in Truro and the associated growth in curb cuts, the Board of Selectmen has established the following Curb Cut Policy in order to address inherent safety concerns.

This policy is intended to provide control over access to Town or State owned roads and uniformity of requirements and standards of construction for every curb cut request. Upon inspection by the Director of the Department of Public Works, there may be additional construction requirements imposed for a particular situation, but none that would be contradictory to the Subdivision Control Laws as outlined in MGL Chapter 41, Sections 81K through 81GG, or the Town of Truro Rules and Regulations governing the Subdivision of Land (Rules and Regulations), Sections 3.6.2, 3.6.6, 4, Table 1 and Section 1.5.

2. Policy

Alteration of existing curb cut(s) and/or requests for additional curb cuts off of a Town or State owned road(s) shall cause an applicant to for file a Curb Cut Permit (CCP). Any application for a building permit that includes a proposed curb cut on property off a Town or State owned road will first require an approved CCP. The approved CCP must be provided to the Truro Building Commissioner prior to or at the time of requesting a building permit. No such building permit will be issued without an approved CCP. Additionally, a final certificate of occupancy for the construction will not be issued unless the conditions of the CCP have been met.

The Truro Board of Selectmen will refer any Town concerns regarding proposed curb cuts on State owned roads to the Massachusetts Highway Department for consideration.

The curb cut construction requirements of this Policy will be applicable to new construction, existing structures, and renovations thereto.

3. Action

Application for a CCP will be made on approved forms available at Town Hall or the Department of Public Works. A copy of the current (as of this date) CCP application form is attached as Exhibit 1. The applicant for

Selectmen's Policy #28 Curb Cut Policy Page 2

a CCP, or his/her agent, will be available to the Director of the Department of Public Works and the Chief of Police to enable a site inspection and to answer any questions regarding the CCP application.

The Planning Board approval/sign off is required for approved subdivision roads on Town or State roads and for endorsed Site Plan Review on Town or State roads.

All curb cuts shall be located and constructed in such a manner so as to **preclude**:

- a. Damage to the Town or State road either at the time of construction or in the future;
- b. Drainage from private property onto the Town or State road;
- c. Introduction of sand, soils, or other materials onto the Town or State road; and
- d. Any other potential hazard to public safety as may be identified by the Director of the Department of Public Works and/or the Chief of Police.

All curb cuts will comply with the Town of Truro construction requirements, as noted on the attached information sheet and shown as Exhibit 2; the design standards shown under the Rules and Regulations, Section 2.5.8; the Mass Highway permit requirements as applicable; and/or as required by the Director of the Department of Public Works.

All applications for a curb cut and approval of performance conditions on Town roads shall be subject to review, including a site visit by the Director of the Department of Public Works and the Chief of Police, prior to approval. The Director shall make recommendations on each application, based upon the Town's construction requirements as outlined above, such as location, materials to be used, catch basin(s) location(s), and so forth, if required. All such required construction will be at the applicant's expense. The Chief of Police will review the application site to ascertain that the curb cut will not be detrimental to traffic flow and the public's safety.

Final approval by the Director of the Department of Public Works shall be made only after approval by the Planning Board, if required, after completion of all construction, and after a final inspection by the Director of the Department of Public Works has been made. Final written approval shall become a part of the property records maintained by the Building Commissioner, and shall be completed prior to the issuance of a certificate of occupancy.

The Board of Selectmen may waive any requirements of this policy, at their sole discretion, when such waiver is deemed to be in the best interests of, and at no cost to, the Town of Truro.

4. Enforcement

Failure to comply with this policy shall result in one or more of the following actions:

a. A refusal to issue a building permit (permit approval) and/or a certificate of occupancy (permit compliance);

Selectmen's Policy #28 Curb Cut Policy Page 3

- b. A request to Mass Highway for disapproval of the applicant's request for a permit to enter a State Highway; and/or
- c. A penalty of \$300.00 for each violation through the non-criminal disposition process as outlined in the Truro General Bylaws. Each day a violation exists shall be considered a new violation.

2. Process

Following is an outline of the chronological process to be used for conformance to this Policy:

- a. Applicant submits an approved application for a Curb Cut Permit.
- b. Director of the Department of Public Works performs a site visit, attaches his recommendations to the Board of Selectmen, and forwards the completed curb cut application to the Chief of Police.
- c. The Chief of Police performs a site visit; he notes his approval/disapproval of the application based on safety considerations and forwards the application to the Board of Selectmen.
- d. Board of Selectmen approves/disapproves the application w/wo conditions and forwards the results to the applicant. If the application is disapproved, the process starts over again with a revised application reflecting the reason(s) for disapproval.
- e. Upon the approval of the Board of Selectmen, applicants whose curb cut applications are tied to a building permit will proceed as below:
 - 1. Applicant includes the approved Curb Cut Permit to his/her application for a building permit.
 - 2. Construction occurs.
 - 3. Property owner or his/her agent applies for a certificate of occupancy.
 - 4. Director of the Department of Public Works performs a site visit to determine compliance with the conditions of the Curb Cut Permit and informs the Building Commissioner, in writing, that the conditions have or have not been met. If the latter, the applicant will be informed of what actions are required to meet the conditions of the Curb Cut Permit and that they must be completed prior to the issuance of a certificate of occupancy.

| Alfred Gaechter, Chairman | Gary Palmer, Vice-Chairman | _ |
|----------------------------|---|---|
| Christopher R. Lucy, Clerk | Curtis Hartman | |
| _ | Janet W. Worthington Board of Selectmen | |
| | Town of Truro | |

EXHIBIT 2

TOWN OF TRURO CURB CUT DESIGN AND CONSTRUCTION REQUIREMENTS

<u>General:</u> Any owner of property abutting Town or State roads shall, before beginning any construction, make written application to the Board of Selectmen, in duplicate. The application will be accompanied by a plan showing the following:

- 1. Complete plans drawn to scale on the property in question, including the location of property lines and all existing driveways, using a scale of no less than 40' = 1''.
- 2. Indication of any drive that is to be altered or closed.

The following additional requirements must be met and agreed upon by the applicant/owner:

- 1. The applicant must furnish a list of all materials, including any necessary signs, to be part of any construction within the Town or State layout.
- 2. All work and material shall meet the standards of the Town of Truro and/or the Mass Highway requirements, if applicable.
- 3. Any alterations to the original application shall require a new permit.
- 4. All curb cuts and street approaches will be inspected during and after construction, and the Town has the right to stop work until such time as any objectionable conditions are corrected at the applicant/owner's expense.
- 5. The cost of any/all construction and maintenance of any work to take place within the Town or State layout; all materials and labor; and any work specified and approved by the Board of Selectmen, shall be borne by the applicant/owner, their grantees, successors and assignees.

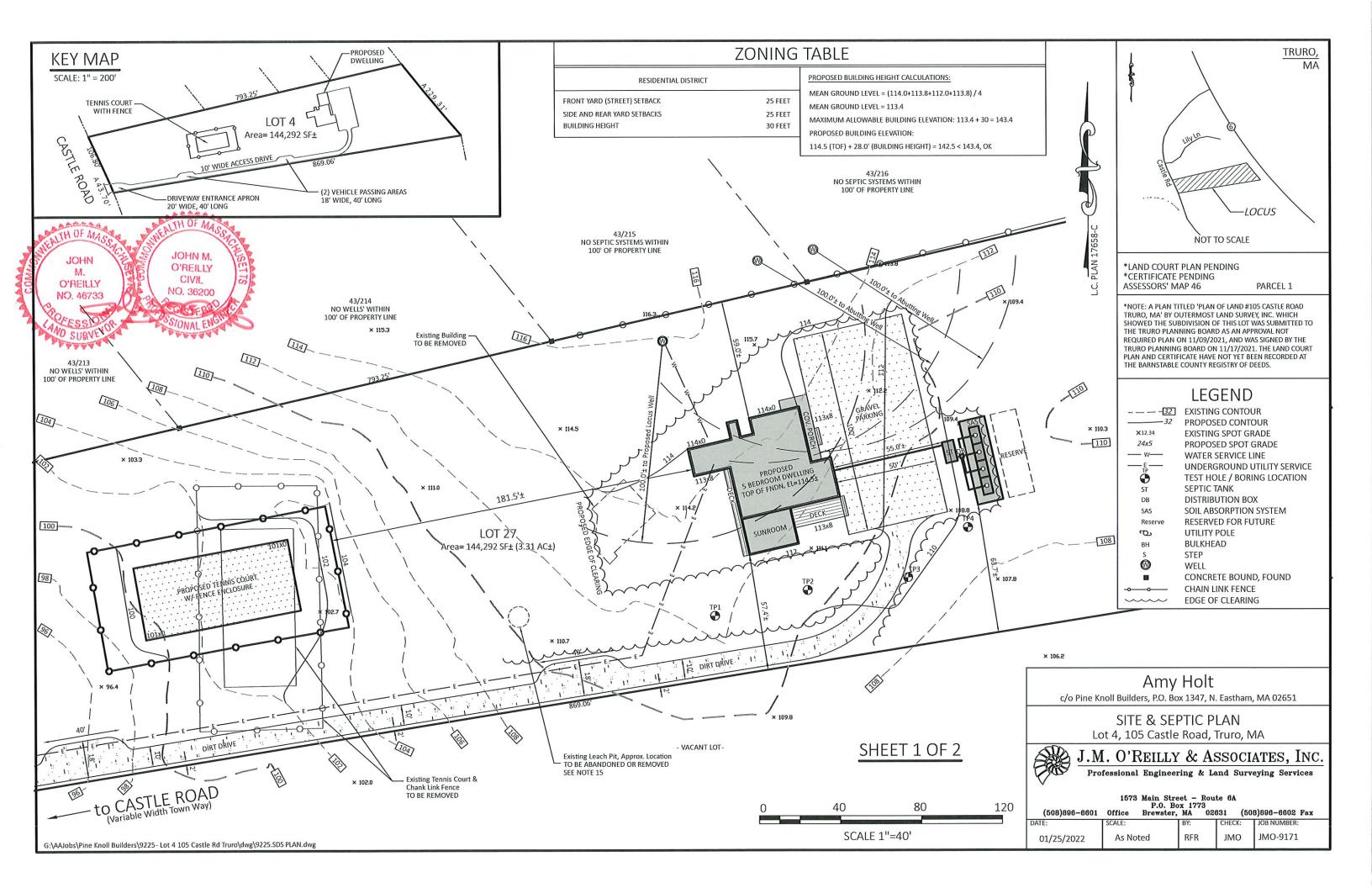
Design and Construction Requirements:

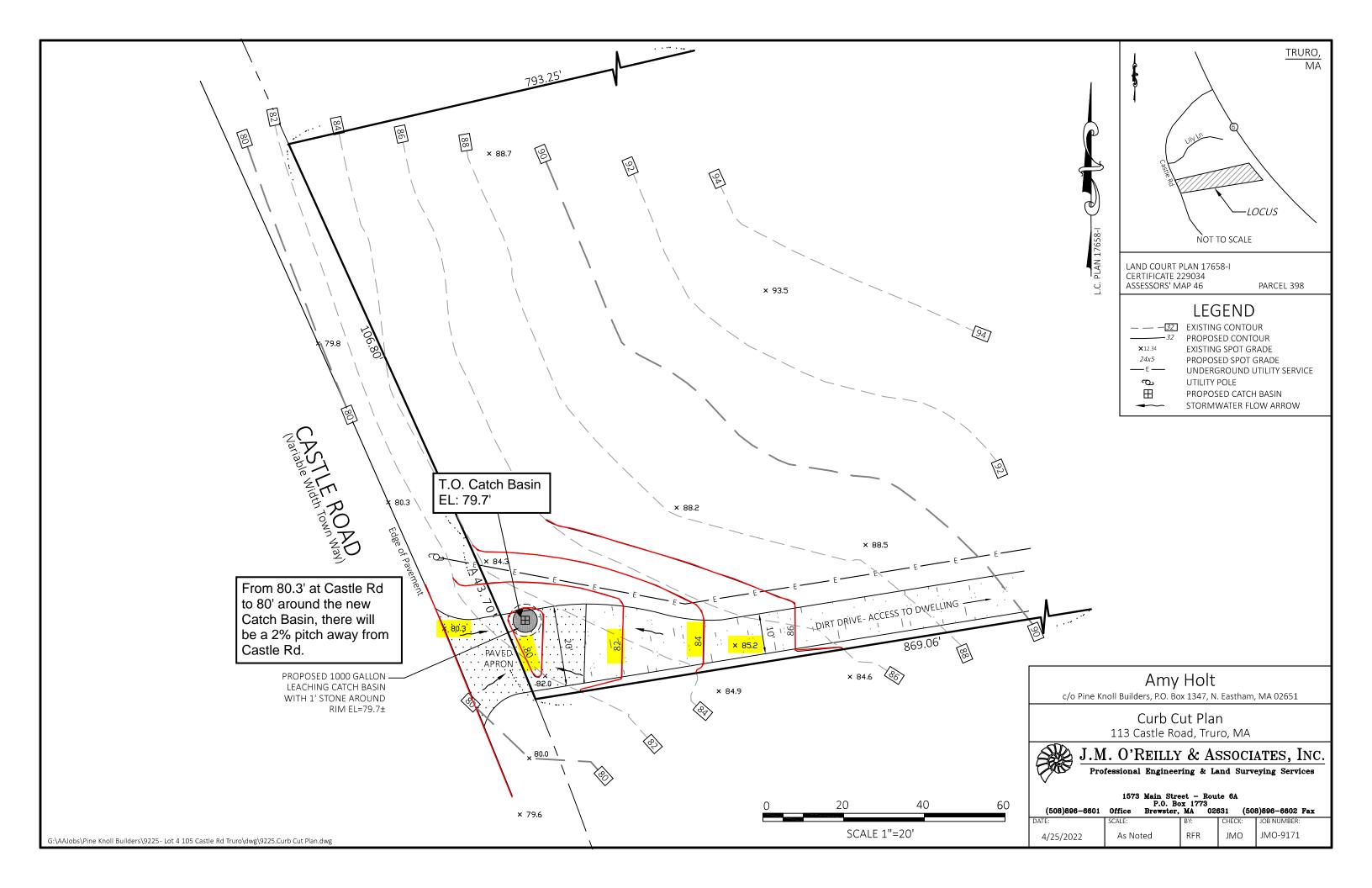
Driveways should be located to the best advantage with regard to the road alignment, profile, sight distance conditions, road safety, and so forth.

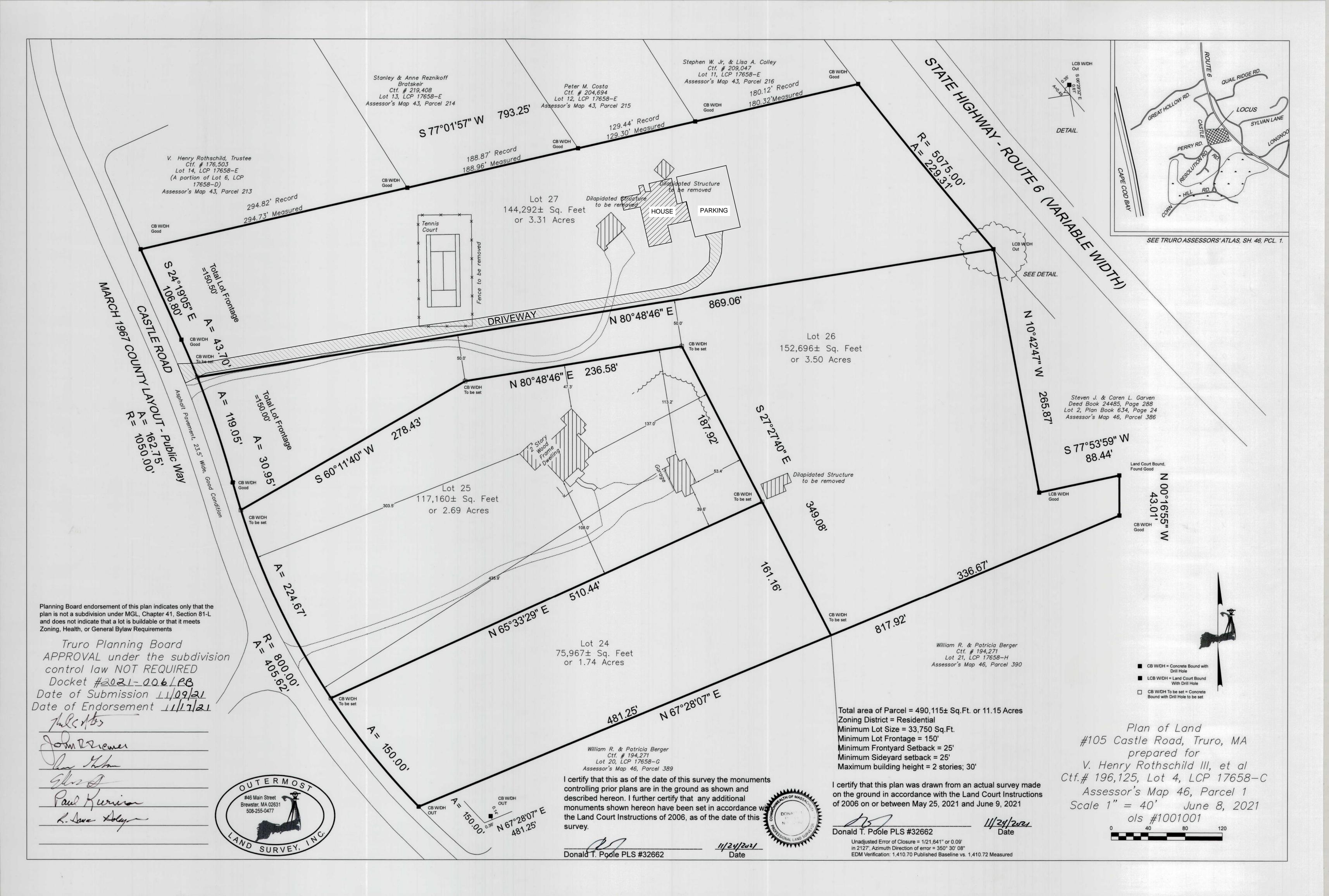
The standards call for not more than one (1) curb cut for any one property. A variance may be granted by the Board of Selectmen, subject to an individual need.

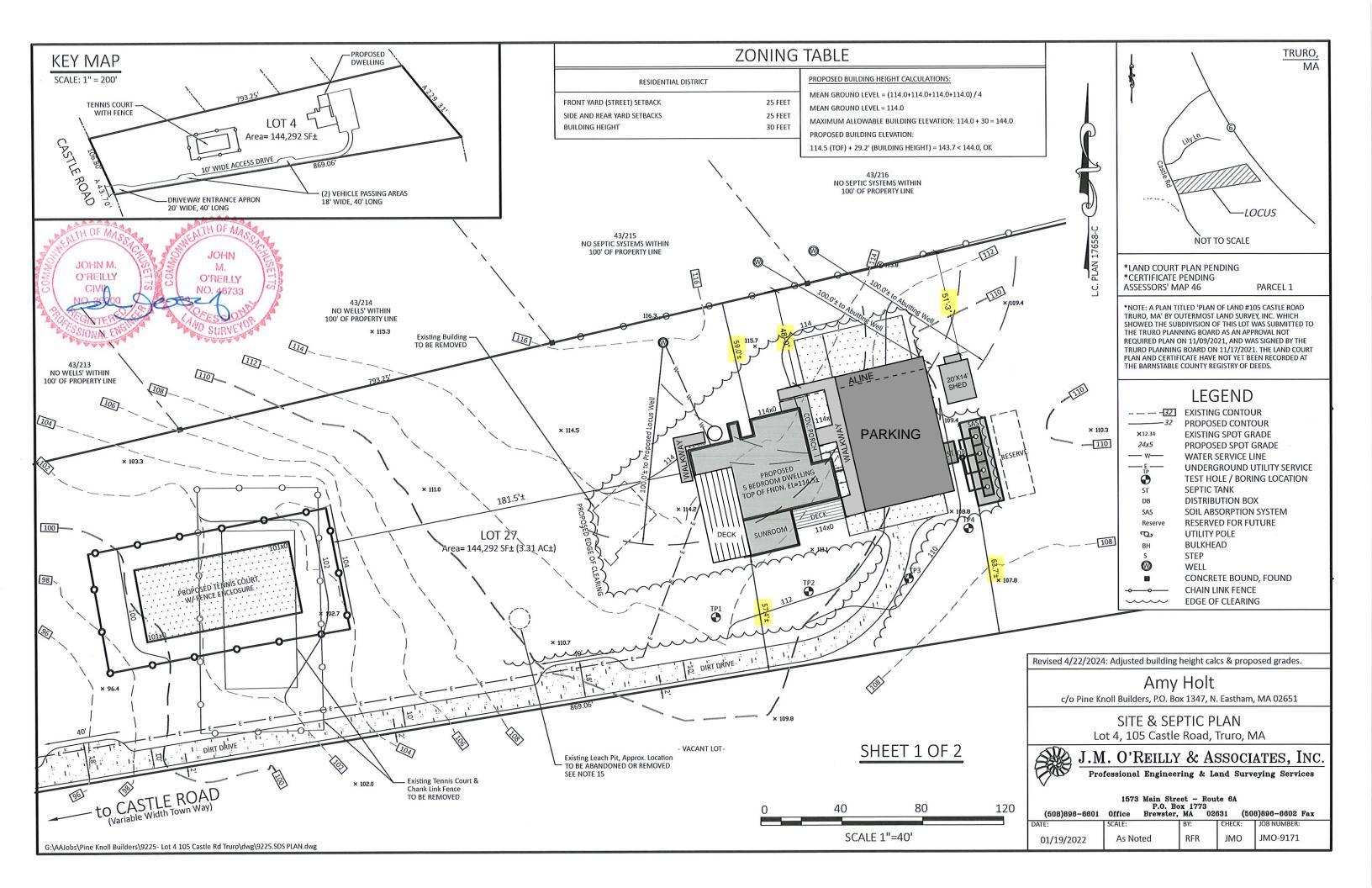
The radius of a private driveway may not extend beyond the private owner's property line without the abutting owner's written consent.

All driveways or private road entrances or exits shall be hot mixed and bermed, oiled, or hardened with such materials to the road/property sideline so as to prevent erosion of such driveway/private road entrance or exit which would cause sand or material to be washed onto Town or State roads. This should be completed as soon as possible, weather permitting.









GENERAL NOTES:

A.) NEITHER DRIVEWAYS NOR PARKING AREAS ARE ALLOWED OVER SEPTIC SYSTEM LINLESS H-20 COMPONENTS ARE USED.

B.) THE DESIGNER WILL NOT BE RESPONSIBLE FOR THE SYSTEM AS DESIGNED UN-LESS CONSTRUCTED AS SHOWN. ANY CHANGES SHALL BE APPROVED IN WRITING.

C.) CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING THE LOCATION OF ALL UNDERGROUND AND OVERHEAD UTILITIES PRIOR TO COMMENCEMENT OF WORK.

CONSTRUCTION NOTES:

- 1.) ALL CONSTRUCTION SHALL CONFORM TO THE STATE ENVIRONMENTAL CODE, TITLE 5. AND THE REQUIREMENTS OF THE LOCAL BOARD OF HEALTH.
- 2.) SEPTIC TANK(S), GREASE TRAP(S), DOSING CHAMBER(S) AND DISTRIBUTION BOX(ES) SHALL BE SET ON A LEVEL STABLE BASE WHICH HAS BEEN MECHANICALLY COMPACTED, OR ON A 6 INCH CRUSHED STONE BASE.
- 3.) SEPTIC TANK(S) SHALL MEET ASTM STANDARD C1127-93 AND SHALL HAVE AT LEAST THREE 20" DIAMETER MANHOLES. THE MINIMUM DEPTH FROM THE BOT-TOM OF THE SEPTIC TANK TO THE FLOW LINE SHALL BE 48".
- 4.) SCHEDULE 40 PVC INLET AND OUTLET TEES SHALL EXTEND A MINIMUM OF 6" ABOVE THE FLOW LINE OF THE SEPTIC TANK AND SHALL BE INSTALLED ON THE CENTERLINE OF THE TANK DIRECTLY UNDER THE CLEANOUT MANHOLE.
- 5.) RAISE COVERS OF THE SEPTIC TANK AND DISTRIBUTION BOX WITH PRECAST CONCRETE WATER TIGHT RISERS OVER INLET AND OUTLET TEES TO WITHIN 6" OF FINISH GRADE, OR AS APPROVED BY THE LOCAL BOARD OF HEALTH AGENT.
- 6.) PIPING SHALL CONSIST OF 4" SCHEDULE 40 PVC OR EQUIVALENT. PIPE SHALL BE LAID ON A MINIMUM CONTINUOUS GRADE OF NOT LESS THAN 1%.
- 7.) DISTRIBUTION LINES FOR SOIL ABSORPTION SYSTEM (AS REQUIRED) SHALL BE 4" DIAMETER SCHEDULE 40 PVC LAID AT 0.005 FT/FT. LINE SHALL BE CAPPED
- 8.) OUTLET PIPES FROM DISTRIBUTION BOX SHALL REMAIN LEVEL FOR AT LEAST 2' BEFORE PITCHING TO SOIL ABSORPTION SYSTEM. WATER TEST DISTRIBUTION BOX TO ASSURE EVEN DISTRIBUTION.
- 9.) DISTRIBUTION BOX SHALL HAVE A MINIMUM SUMP OF 6" MEASURED BELOW THE OUTLET INVERT.
- 10.) BASE AGGREGATE FOR THE LEACHING FACILITY SHALL CONSIST OF 3/4" TO 1-1/2" DOUBLE WASHED STONE FREE OF IRON, FINES AND DUST AND SHALL BE INSTALLED BELOW THE CROWN OF THE DISTRIBUTION LINE TO THE BOTTOM OF THE SOIL ABSORPTION SYSTEM. BASE AGGREGATE SHALL BE COVERED WITH A 2" LAYER OF 1/8" TO 1/2" DOUBLE WASHED STONE FREE OF IRON, FINES AND DUST.
- 11.) VENT SOIL ABSORPTION SYSTEM WHEN DISTRIBUTION LINES EXCEED 50 FEET: WHEN LOCATED EITHER IN WHOLE OR IN PART UNDER DRIVEWAYS, PARKING AREAS, TURNING AREAS OR OTHER IMPERVIOUS MATERIAL; OR WHEN PRESSURE DOSED.
- 12.) SOIL ABSORPTION SYSTEM SHALL BE COVERED WITH A MINIMUM OF 9" OF CLEAN MEDIUM SAND (EXCLUDING TOPSOIL).
- 13.) FINISH GRADE SHALL BE A MAXIMUM OF 36" OVER THE TOP OF ALL SYSTEM COMPONENTS, INCLUDING THE SEPTIC TANK, DISTRIBUTION BOX, DOSING CHAMBER AND SOIL ABSORPTION SYSTEM. SEPTIC TANKS SHALL HAVE A MINIMUM COVER
- 14.) FROM THE DATE OF INSTALLATION OF THE SOIL ABSORPTION SYSTEM UNTIL RECEIPT OF A CERTIFICATE OF COMPLIANCE, THE PERIMETER OF THE SOIL ABSORP TION SYSTEM SHALL BE STAKED AND FLAGGED TO PREVENT THE USE OF SUCH AREA FOR ALL ACTIVITIES THAT MIGHT DAMAGE THE SYSTEM.
- 15.) EXISTING CESSPOOL(S) TO BE REMOVED OR ABANDONED IN PLACE IN ACCORDANCE WITH 310 CMR 15.354. CESSPOOL AND ANY CONTAMINATED SOIL WITHIN 5' OF THE PROPOSED SOIL ABSORPTION SYSTEM SHALL BE REMOVED AND REPLACED WITH CLEAN SAND. AREA TO BE COMPACTED TO MINIMIZE SETTLING.
- 16.) INSTALLER SHALL VERIFY BUILDING SEWER INVERT ELEVATION PRIOR TO INSTALLATION OF ANY SEPTIC SYSTEM COMPONENTS.

SYSTEM DESIGN CALCULATIONS:

SEWAGE DESIGN FLOW: 5 BEDROOM DWELLING @ 110 GPD = 550 GPD

LEACHING CAPACITY REQUIRED

5 BEDROOMS (MAX.) @ 110 GPD = 550 GPD REQUIRED

SEPTIC TANK CAPACITY REQUIRED: DAILY FLOW = 550 GPD @ 200% = 1,100 GAL. REQUIRED

SEPTIC TANK CAPACITY PROVIDED:

1500 GALLON SEPTIC TANK (MIN. ALLOWED)

ONE (1) 42.0" X 12.83" X 2.0" LEACHING CHAMBER CAN LEACH: Vt=[(42.0 X 12.83) + (42.0 X 2.0)2 + (12.83 X 2.0)2] X 0.74 GPD/SF=561.05 GPD

561 GPD>550 GPD REQUIRED NOTE: A GARBAGE DISPOSAL IS NOT PERMITTED WITH THIS DESIGN.

ONE (1)- 1500 GALLON SEPTIC TANK
ONE (1)- 6 OUTLET DISTRIBUTION BOX (H-20 RATED)

FOUR (4) - 500 GALLON LEACH CHAMBERS WITH 4' OF STONE ALL AROUND

SOIL TEST LOGS:

| DEPTH FROM SURFACE (INCHES) | SOIL HORIZON | SOIL TEXTURE (USDA) | SOIL COLOR (MUNSELL) | MOTTLING | OTHER |
|-----------------------------------|-----------------|---------------------------|----------------------------|----------|-----------------------|
| 0-9 | Α | Loamy Sand | 10YR3/2 | NONE | |
| 9-21 | В | Loamy Sand | 10YR7/6 | NONE | |
| 21-156 | C1 | Medium-Coarse Sand | 10YR6/4 | NONE | PERC @ 39"; <2 MIN/IN |

TEST HOLE 2: EL=110,4±

| | SOIL HORIZON | SOIL TEXTURE (USDA) | | MOTTLING | OTHER |
|--------|-----------------|---------------------------|---------|----------|-------|
| 0-10 | Α | Loamy Sand | 10YR3/2 | NONE | |
| 10-28 | В | Loamy Sand | 10YR7/6 | NONE | |
| 28-120 | C1 | Medium-Coarse Sand | 10YR6/4 | NONE | |

DATE OF TESTING: 11/30/2021

PERCOLATION RATE: LESS THAN 2 MIN/INCH IN 'C1' LAYER.
WITNESSED BY: ROBERT REEDY, P.E., J.M. O'REILLY & ASSOCIATES, INC.

AROZANA DAVIS, TRURO HEALTH DEPARTMENT

NO WATER ENCOUNTERED

USE A LOADING RATE OF 0.74 GPD/SF FOR SIZING OF SOIL ABSORPTION SYSTEM.

TEST HOLE 3: EL=109.2±

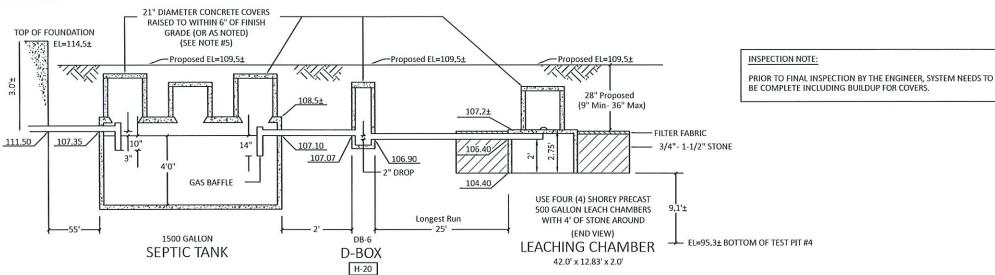
| DEPTH FROM SURFACE (INCHES) | SOIL HORIZON | SOIL TEXTURE (USDA) | SOIL COLOR (MUNSELL) | MOTTLING | OTHER |
|-----------------------------------|-----------------|---------------------------|----------------------------|----------|-----------------------|
| 0-10 | Α | Loamy Sand | 10YR3/2 | NONE | |
| 10-28 | В | Loamy Sand | 10YR7/6 | NONE | |
| 28-127 | C1 | Medium-Coarse Sand | 10YR6/4 | NONE | PERC @ 46"; <2 MIN/IN |

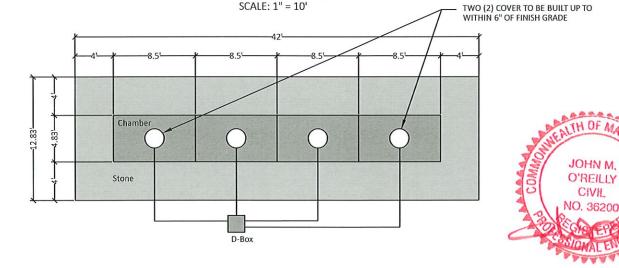
TEST HOLE 4: FL=108 6+

| TEST HOLE I. E | 100107 | | | | |
|-----------------------------------|-----------------|---------------------------|---------|----------|-------|
| DEPTH FROM SURFACE (INCHES) | SOIL HORIZON | SOIL TEXTURE (USDA) | | MOTTLING | OTHER |
| 0-9 | Α | Loamy Sand | 10YR3/2 | NONE | |
| 9-24 | В | Loamy Sand | 10YR7/6 | NONE | |
| 24-160 | C1 | Medium-Coarse Sand | 10YR6/4 | NONE | |

FLOW PROFILE:

NOT TO SCALE





SHEET 2 OF 2

Amy Holt

c/o Pine Knoll Builders, P.O. Box 1347, N. Eastham, MA 02651

SEWAGE DISPOSAL SYSTEM DESIGN & NOTES Lot 4, 105 Castle Road, Truro, MA



J.M. O'REILLY & ASSOCIATES, INC.

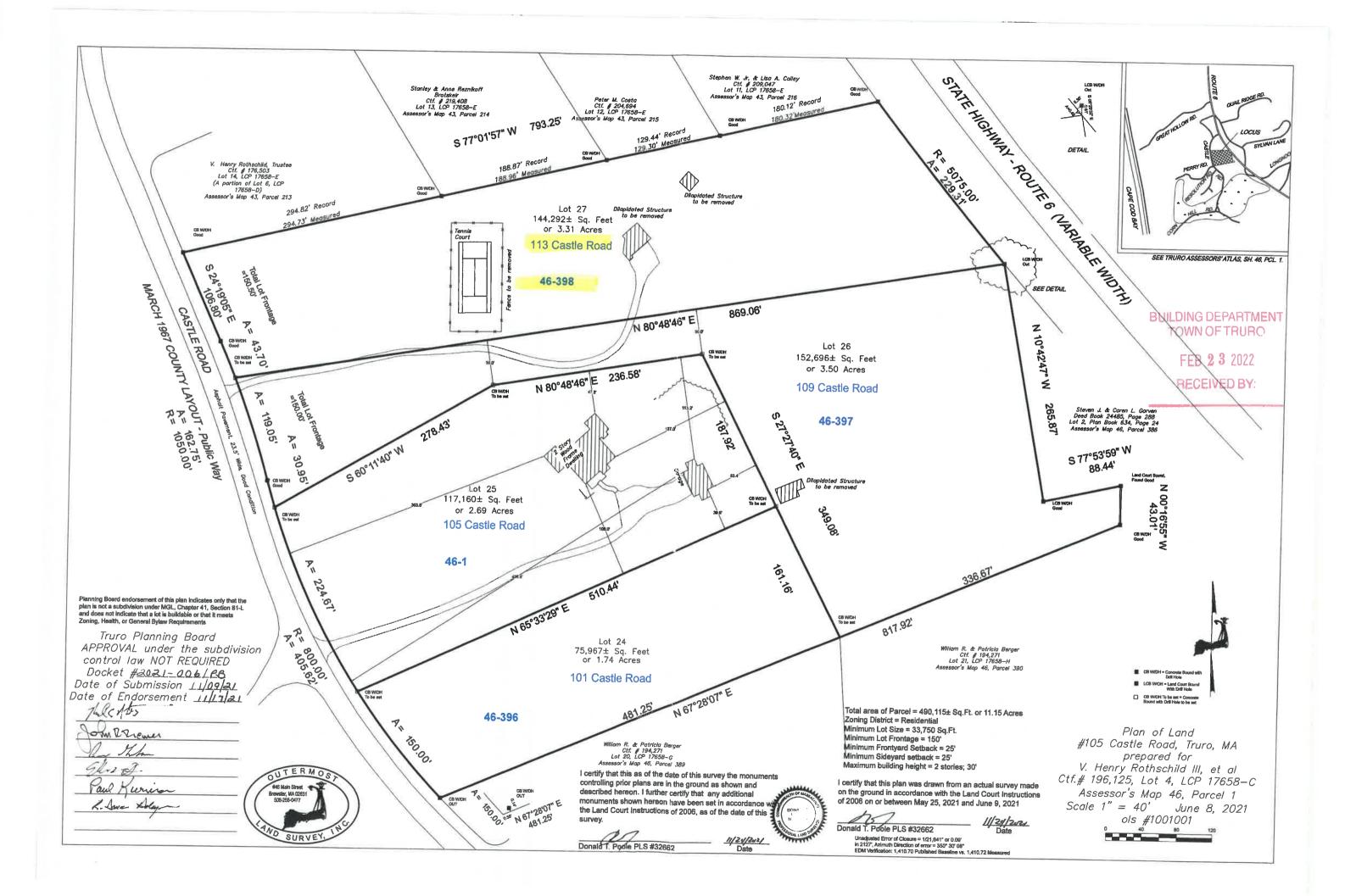
Professional Engineering & Land Surveying Services

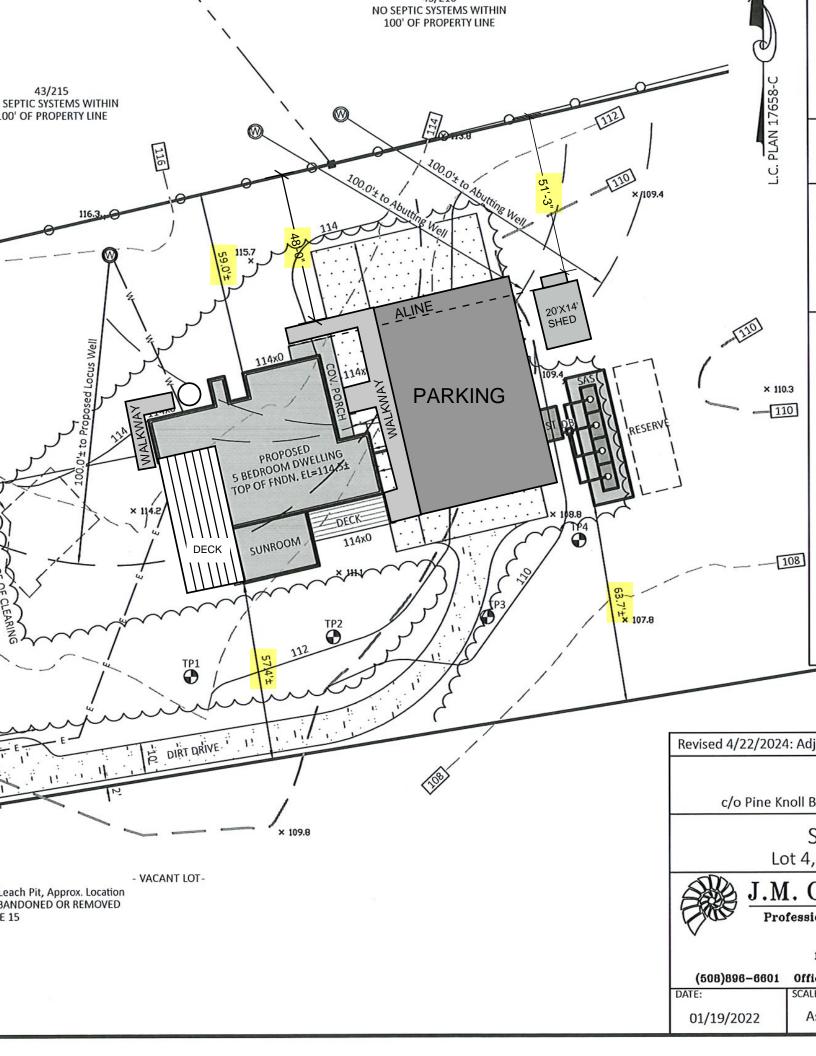
1573 Main Street - Route 6A

P.O. Box 1773 (508)896-6601 Office Brewster, MA 02631 (508)896-8602 Fax

JOB NUMBER: JMO JMO-9171 As Noted 01/25/2022

G.\AAlobs\Pine Knoll Builders\9225- Lot 4 105 Castle Rd Truro\dwg\9225.SDS PLAN.dwg





Pine Knoll 508-255-8292 3 Main Street, Unit 25 N. Eastham, MA 02651

To Whom it may concern,

For the new construction at 113 Castle Rd, Truro, it was realized through the collection of the asbuilt data that the dwelling exceeds Truro's maximum height restrictions by 1'-6". In order to correct this conflict, the truss system has to be adjusted so that the ridge height is lowered by at least 1'-6". Within this packet is the engineered stamped truss adjustments lowering the ridge height by 1'-9". This will put the new height 3" below Truro's maximum height.

It is the intent of the homeowner and the general contractor to complete this correction as soon as possible. Due to the availability of labor, this work cannot be done before the summer, so the intent is to complete this correction by the end of the year 2024.

The original intent was for the house to be ready for move-in by this summer. Since the corrective work will delay the final CO until after summer, it is the general contractor's intent to complete all work except for final energy sign off and the final CO and receive a partial CO so that the homeowner can be in the house for the summer. Once summer is over and the house is empty, the corrective work would take place before the end of the year.

Amy Holt (Honfe owner)

Paul Holt (Home owner)

John Ferro (General Contractor)

4/23/24

Commonwealth of Massachusetts

Country of Bainstable

Commonwealth of Masser Tell, Paul, Holt, and on 4/23/24 before me the undersigned Army Holt, Paul, Holt, and John Ferro proved to me by Ma beense swere to me that they signed the document as their free act and deed.

Kaser a. La Voie Comm: 12/29/28 Karen a. La Voie Comm: 12/29/28

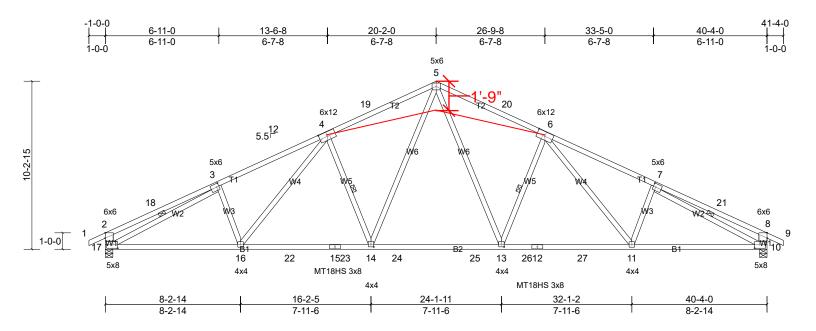
| Job | Truss | Truss Type | Qty | Ply | 105 Castle Rd |
|--------|-------|------------|-----|-----|--------------------------|
| Q15203 | Т3 | Common | 6 | 1 | Job Reference (optional) |

Stark Truss Company, Inc., North Kingstown, RI 02852

Run: 8.53 S May 4 2022 Print: 8.530 S May 4 2022 MiTek Industries, Inc. Thu Jul 21 14:52:50

 $ID: GG1Z2dJD5mJWpbKpN24AoazYR89-ORw7mj7CIDjxbNEeuavOrMHdG_yUUh_yAkw3jXyvq3x$

Page: 1



Scale = 1:70.2

Plate Offsets (X, Y): [2:0-3-14,Edge], [3:0-1-12,0-2-8], [4:0-6-0,0-3-4], [6:0-6-0,0-3-4], [7:0-1-12,0-2-8], [8:0-3-14,Edge], [10:0-3-8,0-2-12], [13:0-2-0,0-1-12], [14:0-2-0,0-1-12], [17:0-3-8,0-2-12]

| | | | _ | | | | | | | | | _ |
|-------------|-------|-----------------|-----------------|------------|------|----------|-------|-------|--------|-----|----------------|----------|
| Loading | (psf) | Spacing | 2-0-0 | CSI | | DEFL | in | (loc) | I/defl | L/d | PLATES | GRIP |
| TCLL (roof) | 30.0 | Plate Grip DOL | 1.15 | TC | 0.93 | Vert(LL) | -0.30 | 14-16 | >999 | 360 | MT20 | 197/144 |
| Snow (Pf) | 35.0 | Lumber DOL | 1.15 | BC | 0.99 | Vert(CT) | -0.51 | 11-13 | >938 | 240 | MT18HS | 197/144 |
| TCDL | 15.0 | Rep Stress Incr | YES | WB | 0.93 | Horz(CT) | 0.20 | 10 | n/a | n/a | | |
| BCLL | 0.0* | Code | IRC2015/TPI2014 | Matrix-MSH | | | | | | | | |
| BCDL | 10.0 | | | | | | | | | | Weight: 186 lb | FT = 20% |

LUMBER

TOP CHORD 2x4 SPF 2100F 1.8E *Except* T1:2x4 SPF 1650F 1.5E BOT CHORD 2x4 SPF No.2 *Except* B2:2x4 SPF 1650F 1.5E WEBS 2x4 SPF No.2 *Except* W1:2x6 SPF 1650F 1.5E

REACTIONS (lb/size) 10=2515/0-5-8, (min. 0-3-15), 17=2515/0-5-8, (min. 0-3-15)

Max Horiz 17=158 (LC 20)

Max Uplift 10=-328 (LC 17), 17=-328 (LC 16)

BRACING

TOP CHORD BOT CHORD

WEBS

Structural wood sheathing directly applied, except end verticals. Rigid ceiling directly applied or 2-2-0 oc bracing, Except: 10-0-0 oc bracing: 13-14.

10-0-0 oc bracing: 13-1 1 Row at midpt

6-13, 4-14, 3-17, 7-10

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-18=-856/220, 3-18=-705/239, 3-4=-3886/558, 4-19=-3233/511, 5-19=-3081/531, 5-20=-3081/531, 6-20=-3233/511,

 $6\text{-}7\text{=-}3886/558, \, 7\text{-}21\text{=-}705/238, \, 8\text{-}21\text{=-}856/220, \, 2\text{-}17\text{=-}779/276, \, 8\text{-}10\text{=-}779/276}$

16-17=-518/3499, 16-22=-359/3123, 15-22=-359/3123, 15-23=-359/3123, 14-23=-359/3123, 14-24=-145/2408, 24-25=-145/2408, 13-25=-145/2408, 13-26=-231/3123, 12-26=-231/3123, 12-27=-231/3123, 11-27=-231/3123, 12-26=-231/3123, 12-27=

10-11=-360/3499

5-13=-240/1230, 6-13=-1053/340, 6-11=-133/513, 7-11=-270/223, 5-14=-240/1230, 4-14=-1053/340, 4-16=-133/513,

3-16=-270/223, 3-17=-3316/283, 7-10=-3316/283

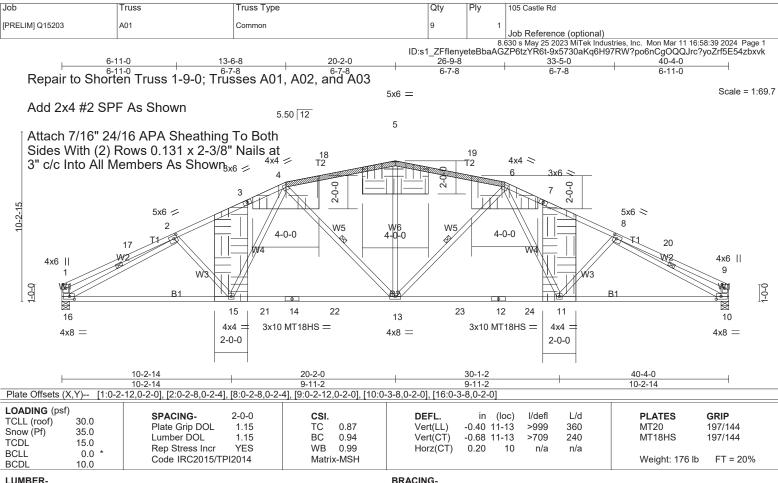
WEBS NOTES

BOT CHORD

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=111mph; TCDL=6.0psf; BCDL=6.0psf; BCDL=6.0psf; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) -1-0-0 to 3-0-6, Interior (1) 3-0-6 to 20-2-0, Exterior (2) 20-2-0 to 24-2-6, Interior (1) 24-2-6 to 41-4-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pf=35.0 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 35.0 psf on overhangs non-concurrent with other live loads.
- 6) All plates are MT20 plates unless otherwise indicated.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 328 lb uplift at joint 17 and 328 lb uplift at joint 10.
- 0) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



TOP CHORD BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SPF 2100F 1.8E *Except*

T1: 2x4 SPF No.2 BOT CHORD 2x4 SPF 1650F 1.5E

2x4 SPF No.2 *Except* **WEBS**

W1: 2x6 SPF 1650F 1.5E

REACTIONS. (lb/size) 16=2393/0-5-8 (min. 0-3-12), 10=2392/0-5-8 (min. 0-3-12)

Max Horz 16=147(LC 20)

Max Uplift 16=-295(LC 16), 10=-295(LC 17)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-17=-677/130, 2-17=-509/149, 2-3=-3793/486, 3-4=-3541/516, 4-18=-2832/455, TOP CHORD

5-18=-2681/475, 5-19=-2681/475, 6-19=-2832/455, 6-7=-3541/516, 7-8=-3793/486,

8-20=-509/149, 9-20=-677/131, 1-16=-546/166, 9-10=-546/166

15-16=-549/3518, 15-21=-369/3125, 14-21=-369/3125, 14-22=-369/3125, BOT CHORD 13-22=-369/3125, 13-23=-276/3125, 12-23=-276/3125, 12-24=-276/3125,

11-24=-276/3125, 10-11=-404/3518

2-16=-3499/401, 8-10=-3499/401, 4-15=-51/489, 2-15=-313/236, 4-13=-1171/326, **WEBS**

5-13=-194/1635, 6-13=-1171/326, 6-11=-51/489, 8-11=-313/236

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=140mph Vasd=111mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-2-12 to 4-3-2, Interior(1) 4-3-2 to 20-2-0, Exterior(2) 20-2-0 to 24-2-6, Interior(1) 24-2-6 to 40-1-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pf=35.0 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 295 lb uplift at joint 16 and 295 lb uplift at joint
- 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



Structural wood sheathing directly applied, except end verticals.

installed during truss erection, in accordance with Stabilizer

2-16, 8-10, 4-13, 6-13

MiTek recommends that Stabilizers and required cross bracing be

Rigid ceiling directly applied or 2-2-0 oc bracing.

1 Row at midpt

Installation guide

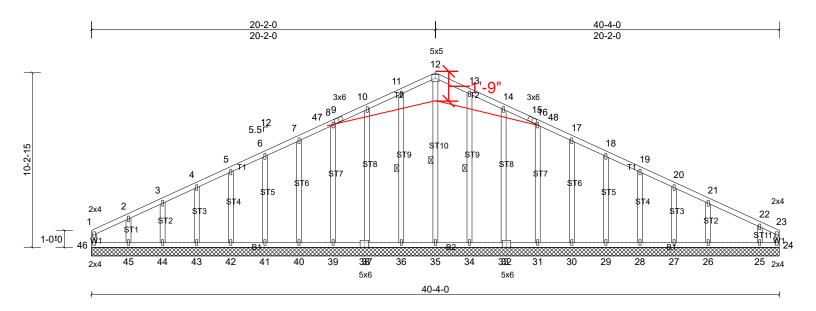
03/12/2024 1 of 4

| Job | | Truss | Truss Type | Qty | Ply | 105 Castle Rd |
|--------|---|-------|------------------------|-----|-----|--------------------------|
| Q15203 | 3 | T2GE | Common Supported Gable | 1 | 1 | Job Reference (optional) |

Stark Truss Company, Inc., North Kingstown, RI 02852

Run: 8.53 S Jan 25 2022 Print: 8.530 S May 4 2022 MiTek Industries, Inc. Thu Jul 21 14:52:49

ID:KcON374IZwwD1gNAmZFjXazYR8S-wFNIYN6aXvb5 DfSKtO9J9kf7bqSlNyox4BWB5yvq3y



Scale = 1:67.6

| Loading | (psf) | Spacing | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|-------------|-------|-----------------|-----------------|-----------|------|-----------|------|-------|--------|-----|----------------|----------|
| TCLL (roof) | 30.0 | Plate Grip DOL | 1.15 | TC | 0.12 | Vert(LL) | n/a | - | n/a | 999 | MT20 | 197/144 |
| Snow (Pf) | 35.0 | Lumber DOL | 1.15 | BC | 0.08 | Vert(TL) | n/a | - | n/a | 999 | | |
| TCDL | 15.0 | Rep Stress Incr | YES | WB | 0.34 | Horiz(TL) | 0.01 | 24 | n/a | n/a | | |
| BCLL | 0.0* | Code | IRC2015/TPI2014 | Matrix-MR | | | | | | | | |
| BCDL | 10.0 | [| | | | | | | | | Weight: 213 lb | FT = 20% |

LUMBER TOP CHORD 2x4 SPF No.2 **BOT CHORD** 2x4 SPF No.2 WEBS 2x4 SPF No.2 **OTHERS** 2x4 SPF No.2

REACTIONS All bearings 40-4-0.

(lb) - Max Horiz 46=149 (LC 20)

Max Uplift All uplift 100 (lb) or less at joint(s) 24, 26, 27, 28, 29, 30, 31, 33, 34, 36, 37, 39, 40, 41, 42, 43, 44, 46 except 25=-197 (LC 17), 45=-153 (LC 16)

Max Grav All reactions 250 (lb) or less at joint(s) 24, 27, 28, 29, 30, 40, 41, 42, 43, 44, 46 except 25=309 (LC 23), 26=325 (LC 1), 31=312 (LC 23), 33=333 (LC 23), 34=350 (LC 23), 35=256 (LC 28), 36=350 (LC 22), 37=333 (LC 22), 39=312 (LC 22), 45=265 (LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 6-7=-91/258, 7-47=-109/303, 8-47=-86/308, 8-9=-126/349, 9-10=-113/358, 10-11=-144/411, 11-12=-161/453,

12-13=-161/444, 13-14=-144/401, 14-15=-113/348, 15-16=-126/339, 16-48=-86/299, 17-48=-109/294 **WEBS** 22-25=-262/257, 12-35=-253/29, 11-36=-309/159, 10-37=-292/138, 8-39=-271/94, 13-34=-309/158, 14-33=-292/138,

16-31=-271/94, 21-26=-270/187

NOTES

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=111mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner (3) 0-1-12 to 4-2-0, Exterior (2) 4-2-0 to 20-2-0, Corner (3) 20-2-0 to 24-2-0, Exterior (2) 24-2-0 to 40-2-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pf=35.0 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; 4) Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- All plates are 1.5x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- 8) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2-0-0 oc. 9)
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 10)
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and 11) any other members
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 46, 24, 36, 37, 39, 40, 41, 42, 43, 44, 34, 33, 31, 30, 29, 28, 27, 26 except (jt=lb) 25=196, 45=152.

BRACING

BOT CHORD

WEBS

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing 12-35, 11-36, 13-34 1 Row at midpt

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

Installation guide.

Truss Truss Type Qty 105 Castle Rd [PRELIM] Q15203 A01GE Common Supported Gable Job Reference (optional) R630 s May 25 2023 MTRel Industries, Inc. Mon Mar 11 17:32:15 2024 Page 1 ID:KcON374IZwwD1gNAmZFjXazYR8S-7nvGAMzv2XG6_FeTvCHYwot7r_7MMt6IQJSvsEzbxQE 20-2-0 40-4-0 20-2-0 Repair to Shorten Truss 1-9-0 Scale = 1:72.1 5x5 =5.50 12 Add 2x4 #2 SPF As Shown 4-3-8 13 4-9-4 4-9-4 Attach 7/16" 24/16 APA Sheathing To Both Sides With (2) Rows 0.131 x 2-3/8" Nails at 10 T2 3x6 < 3x6 = 15 3" c/c Into All Members As Shown 48 16 17 6 Š 10-2-15 19 10 20 21 SIT5 ST122 4x5 > 4x5 / **S**Τ SIT4 SITI3 0-0-31 30 38 |46 44 37 36 35 34 33 28 45 42 41 40 32 29 26 25 39 2x4 | | 3x4 =3x4 = 2x4 | II3x6 =3x6 _ 13-6-8 13-6-8 40-4-0 40-4-0 Plate Offsets (X,Y)-- [1:0-1-12,0-2-0], [23:0-1-12,0-2-0], [31:0-2-4,0-1-8], [39:0-2-4,0-1-8] LOADING (psf) SPACING-2-0-0 CSI. DEFL. I/defl L/d **PLATES GRIP** in (loc) TCLL (roof) 30.0 Plate Grip DOL TC 1.15 0.09 Vert(LL) 999 197/144 MT20 n/a n/a Snow (Pf) 35.0 Lumber DOL ВС 0.03 Vert(CT) 999 1.15 n/a n/a **TCDL** 15.0 WB Rep Stress Inci YES 0.34 Horz(CT) 0.01 25 n/a n/a **BCLL** 0.0 Ode IRC2015/TPI2014 FT = 20%Weight: 219 lb Matrix-SH

BRACING-

TOP CHORD

BOT CHORD

WFBS

end verticals

1 Row at midpt

Installation guide

LUMBER-

BCDI

Job

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2

10.0

2x6 SPF 1650F 1.5E *Except* **WEBS**

W2: 2x4 SPF No.2

OTHERS 2x4 SPF No.2

REACTIONS. All bearings 40-4-0

Max Horz 46=147(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 46, 36, 37, 38, 40, 41, 42, 43, 44, 34, 33, 32, 30, 29, 28, 27, 26 except 45=-155(LC 16), 25=-132(LC 17) Max Grav All reactions 250 lb or less at joint(s) 46, 24, 35, 40, 41, 42, 43, 44, 30, 29, 28, 27, 26 except 36=349(LC 22), 37=332(LC 22), 38=311(LC 22),

45=254(LC 22), 34=349(LC 23), 33=332(LC 23), 32=311(LC 23), 25=254(LC 23)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

9-10=-103/272, 10-11=-121/324, 11-12=-138/368, 12-13=-138/357, 13-14=-121/307, TOP CHORD

14-15=-103/255

WEBS 11-36=-309/163, 10-37=-292/136, 9-38=-271/95, 13-34=-309/162, 14-33=-292/136,

15-32=-271/95

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=140mph Vasd=111mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-2-12 to 4-2-0, Exterior(2) 4-2-0 to 20-2-0, Corner(3) 20-2-0 to 24-2-0, Exterior(2) 24-2-0 to 40-1-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pf=35.0 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) All plates are 1.5x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.
- 8) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 9) Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 46, 36, 37, 38, 40, 41, 42, 43, 44, 34, 33, 32, 30, 29, 28, 27, 26 except (jt=lb) 45=155, 25=132.
- 13) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced Continuedand AAGH/27PI 1.



Structural wood sheathing directly applied or 6-0-0 oc purlins, except

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

12-35, 11-36, 13-34

Rigid ceiling directly applied or 10-0-0 oc bracing.

03/12/2024

| Job | Truss | Truss Type | Qty | Ply | 105 Castle Rd |
|--------|-------|-------------------------|-----|-----|--------------------------|
| Q15203 | T1 | Common Structural Gable | 1 | 1 | Job Reference (optional) |

Stark Truss Company, Inc., North Kingstown, RI 02852

Run: 8.53 S May 4 2022 Print: 8.530 S May 4 2022 MiTek Industries, Inc. Thu Jul 21 14:52:47

ID:ke0fYXirtP0cpvWCSYBSm1zYR8x-WghcwL4hE_DW7mwtfkqShW6ybNeDYw0MF6ysbmyvq4?

Structural wood sheathing directly applied or 3-9-7 oc purlins,

MiTek recommends that Stabilizers and required cross bracing be

installed during truss erection, in accordance with Stabilizer

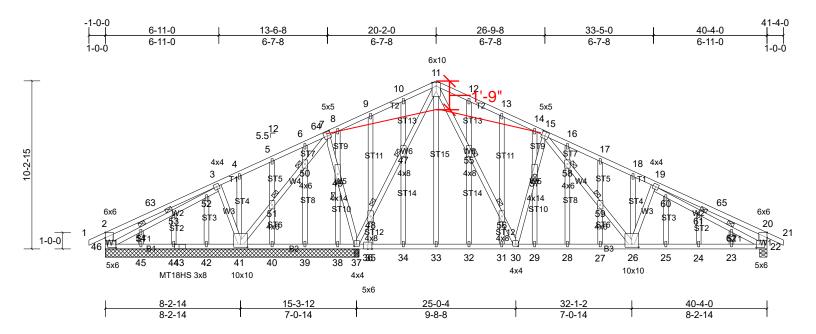
Rigid ceiling directly applied or 6-0-0 oc bracing.

except end verticals.

Installation guide.

61.62

1 Brace at Jt(s): 47, 48, 49, 50, 51, 53, 54, 55, 56, 57, 58, 59,



Scale = 1:70.2

[2:0-3-14, Edge], [7:0-2-8,0-3-0], [15:0-2-8,0-3-0], [20:0-3-14, Edge], [22:0-2-4,0-2-8], [26:0-5-0,0-2-4], [30:0-1-12,0-1-8], [37:0-1-12,0-1-8], [41:0-5-0,0-2-4], [40:0-5Plate Offsets (X, Y): [43:0-3-4,0-1-8], [46:0-2-4,0-2-8]

| | / 6 | | | | | | | <i>(</i> 1) | | | | |
|-------------|-------|-----------------|-----------------|------------|------|----------|-------|--------------|--------|-----|----------------|----------|
| Loading | (psf) | Spacing | 2-0-0 | CSI | | DEFL | ın | (loc) | l/defl | L/d | PLATES | GRIP |
| TCLL (roof) | 30.0 | Plate Grip DOL | 1.15 | TC | 0.84 | Vert(LL) | -0.12 | 27-28 | >999 | 360 | MT20 | 197/144 |
| Snow (Pf) | 35.0 | Lumber DOL | 1.15 | BC | 0.82 | Vert(CT) | -0.20 | 27-28 | >999 | 240 | MT18HS | 197/144 |
| TCDL | 15.0 | Rep Stress Incr | YES | WB | 0.80 | Horz(CT) | 0.03 | 22 | n/a | n/a | | |
| BCLL | 0.0* | Code | IRC2015/TPI2014 | Matrix-MSH | | | | | | | | |
| BCDL | 10.0 | | | | | | | | | | Weight: 297 lb | FT = 20% |

BOT CHORD

JOINTS

LUMBER **BRACING** TOP CHORD 2x4 SPF No.2 TOP CHORD

BOT CHORD 2x4 SPF No.2 2x4 SPF No.2 *Except* W1:2x6 SPF 1650F 1.5E **WEBS**

OTHERS 2x4 SPF No.2

REACTIONS All bearings 15-5-8. except 22=0-5-8

(lb) - Max Horiz 46=-157 (LC 21)

Max Uplift All uplift 100 (lb) or less at joint(s) 38, 39, 40, 42, 45 except 22=-249 (LC 17), 37=-229 (LC 17), 41=-180 (LC 16), 46=-110

Max Grav All reactions 250 (lb) or less at joint(s) 38, 39, 42, 44, 45 except 22=1482 (LC 24), 37=2540 (LC 1), 40=287 (LC 1), 41=343 (LC

23), 46=467 (LC 23)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-63=-471/209, 3-63=-331/228, 3-4=-2/271, 4-5=0/339, 5-6=0/328, 6-64=0/320, 7-64=0/327, 7-8=0/749, 8-9=0/769,

9-10=0/739, 10-11=0/793, 11-12=-983/397, 12-13=-1073/368, 13-14=-1063/324, 14-15=-1029/294, 15-16=-1741/467, 16-17=-1805/443, 17-18=-1829/403, 18-19=-1851/394, 19-65=-863/234, 20-65=-1016/215, 2-46=-597/274,

20-22=-850/266

40-41=-588/234, 39-40=-588/234, 38-39=-588/234, 37-38=-588/234, 29-30=-26/1095, 28-29=-26/1095, 27-28=-26/1095,

26-27=-26/1095, 25-26=-220/1702, 24-25=-220/1702, 23-24=-220/1702, 22-23=-220/1702

11-55=-326/1479, 55-56=-331/1489, 30-56=-343/1548, 30-57=-794/219, 15-57=-758/210, 15-58=-244/914, 58-59=-227/840, 26-59=-220/824, 19-26=-298/161, 37-48=-2114/171, 47-48=-2003/159, 11-47=-2125/172, 7-49=-315/62,

37-49=-340/68, 41-51=-70/494, 50-51=-70/489, 7-50=-75/527, 3-41=-703/318, 46-54=-194/343, 53-54=-192/339, 52-53=-196/347, 3-52=-231/409, 19-60=-1070/153, 60-61=-1071/115, 61-62=-1057/113, 22-62=-1038/113,

11-33=-28/295, 9-48=-398/95, 35-48=-314/93, 8-49=-257/71, 40-51=-255/105, 13-56=-317/83, 31-56=-383/97

NOTES

WEBS

FORCES

BOT CHORD

Unbalanced roof live loads have been considered for this design. 1)

- Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=111mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C 2) Exterior (2) -1-0-0 to 3-0-6, Interior (1) 3-0-6 to 20-0-10, Exterior (2) 20-0-10 to 24-2-0, Interior (1) 24-2-0 to 41-4-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult 3) qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pf=35.0 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; 4) Partially Exp.; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 35.0 psf on overhangs non-concurrent with other live loads. 6)
- All plates are MT20 plates unless otherwise indicated.

Page: 1

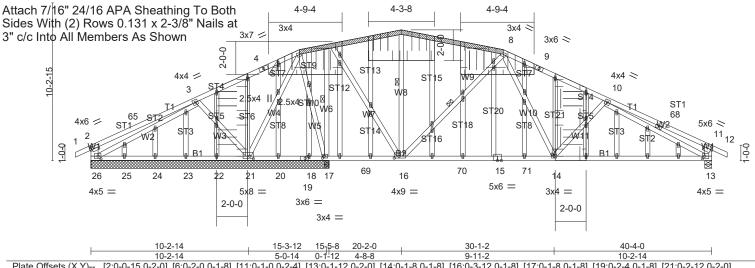
Job Truss Type Qty 105 Castle Rd Truss [PRELIM] Q15203 A03GE Common Structural Gable Job Reference (optional) 8.630 s May 25 2023 MiTek Industries, Inc. Mon Mar 11 21:44:05 2024 Page 1 ID:ke0fYXirtP0cpvWCSYBSm12YR8x-iiwTR?rtd4zAlLmU4ZMvJBh7rwxBfcOMz7fSCRzbtk8 13-6-8 20-2-0 6-11-0 15-3-12 26-9-8 33-5-0 40-4-0 41-4-0 1-0-0 4-10-4

Repair to Shorten Truss 1-9-0

Scale = 1:74.8 5x6 =

Add 2x4 #2 SPF As Shown

5.50 12



[2:0-0-15,0-2-0], [6:0-2-0,0-1-8], [11:0-1-0,0-2-4], [13:0-1-12,0-2-0], [14:0-1-8,0-1-8], [16:0-3-12,0-1-8], [17:0-1-8,0-1-8], [19:0-2-4,0-1-8], [21:0-2-12,0-2-0],Plate Offsets (X,Y)--[26:0-2-4,0-2-0]

| TCLL (roof) Snow (Pf) TCDL | 30.0 35.0 15.0 | Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES | TC 0.84 BC 0.88 WB 0.55 | Vert(LL) -0.32 14-16 >932 360 Vert(CT) -0.50 14-16 >593 240 Horz(CT) 0.04 13 n/a n/a | | RIP 97/144 |
|----------------------------------|----------------------|---|-------------------------------|--|----------------|---------------|
| BCLL BCDL | 0.0 * 10.0 | Code IRC2015/TPI2014 | Matrix-MSH | Horz(CT) 0.04 13 n/a n/a | Weight: 294 lb | FT = 20% |

LUMBER-BRACING-

TOP CHORD 2x4 SPF 1650F 1.5E *Except*

T1: 2x4 SPF No.2 **BOT CHORD** 2x4 SPF No.2

WEBS 2x4 SPF No.2 *Except* W1: 2x6 SPF 1650F 1.5E

OTHERS 2x4 SPF No.2

LOADING (psf)

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied or 3-10-14 oc purlins, except Rigid ceiling directly applied or 6-0-0 oc bracing, Except:

10-0-0 oc bracing: 14-16,13-14.

7-16, 8-16, 10-13, 6-17 1 Row at midpt

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. All bearings 15-5-8 except (jt=length) 13=0-5-8.

Max Horz 26=-158(LC 17) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 18 except 26=-112(LC 16),

13=-243(LC 17), 21=-245(LC 16), 17=-269(LC 17)

Max Grav All reactions 250 lb or less at joint(s) 18, 20, 22, 23, 24, 25 except 26=535(LC 23), 13=1532(LC 24), 21=657(LC 23), 17=2365(LC 1), 17=2365(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD 2-65=-442/193, 3-4=-34/374, 4-5=-12/509, 5-6=0/539, 6-66=-465/205, 7-66=-330/226,

7-67=-383/209, 8-67=-608/188, 8-9=-1519/318, 9-10=-1772/301, 10-68=-592/205,

11-68=-736/186, 2-26=-569/266, 11-13=-715/259

BOT CHORD 20-21=-406/237, 19-20=-406/237, 18-19=-406/237, 17-18=-406/237, 17-69=-510/265,

16-69=-510/265, 16-70=-37/1239, 15-70=-37/1239, 15-71=-37/1239, 14-71=-37/1239,

13-14=-228/1790

WEBS 7-16=-279/14, 8-16=-1286/334, 8-14=-59/642, 10-14=-479/248, 10-13=-1446/154,

3-21=-676/297, 5-17=-499/154, 6-17=-1808/192, 6-16=-132/1611

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=140mph Vasd=111mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -1-0-0 to 3-0-6, Interior(1) 3-0-6 to 20-2-0, Exterior(2) 20-2-0 to 24-2-6, Interior(1) 24-2-6 to 41-4-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pf=35.0 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 35.0 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 1.5x4 MT20 unless otherwise indicated.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Continued on page 2





Stark Truss Company, Inc., North Kingstown, RI 02852

Run: 8.53 S Jan 25 2022 Print: 8.530 S May 4 2022 MiTek Industries, Inc. Thu Jul 21 14:52:54

Structural wood sheathing directly applied or 6-0-0 oc purlins.

installed during truss erection, in accordance with Stabilizer

6-16 MiTek recommends that Stabilizers and required cross bracing be

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt

Installation guide.

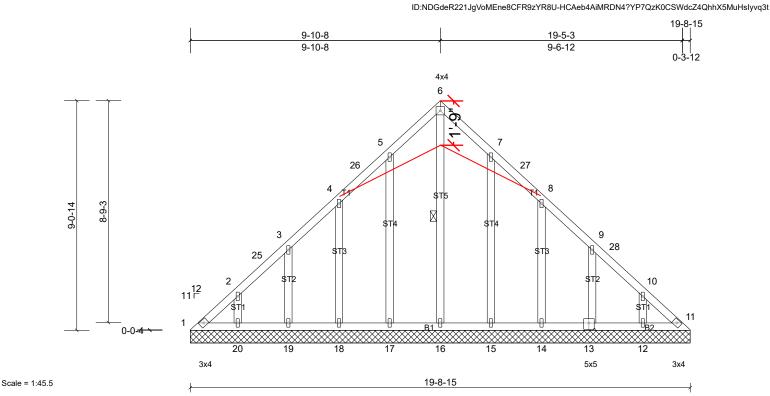


Plate Offsets (X, Y): [13:0-2-8,0-3-0]

| Loading | (psf) | Spacing | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|-------------|-------|-----------------|-----------------|------------|------|-----------|------|-------|--------|-----|---------------|----------|
| TCLL (roof) | 30.0 | Plate Grip DOL | 1.15 | TC | 0.07 | Vert(LL) | n/a | - | n/a | 999 | MT20 | 197/144 |
| Snow (Pf) | 35.0 | Lumber DOL | 1.15 | BC | 0.05 | Vert(TL) | n/a | - | n/a | 999 | | |
| TCDL | 15.0 | Rep Stress Incr | YES | WB | 0.18 | Horiz(TL) | 0.01 | 11 | n/a | n/a | | |
| BCLL | 0.0* | Code | IRC2015/TPI2014 | Matrix-MSH | | | | | | | | |
| BCDL | 10.0 | [| | | | | | | | | Weight: 98 lb | FT = 20% |

BRACING

WEBS

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2x4 SPF No.2 **BOT CHORD** 2x4 SPF No.2 OTHERS 2x4 SPF No.2

REACTIONS All bearings 19-8-15.

(lb) - Max Horiz 1=254 (LC 11)

Max Uplift All uplift 100 (lb) or less at joint(s) 1, 11, 20 except 12=-112 (LC 15), 13=-114 (LC 15), 14=-121 (LC 15), 15=-115 (LC 15), 17=-118 (LC 14), 18=-118 (LC 14), 19=-125 (LC 14)

Max Grav All reactions 250 (lb) or less at joint(s) 1, 11, 12, 13, 14, 18, 19 except 15=261 (LC 22), 16=251 (LC 24), 17=263 (LC 21), 20=267 (LC 21)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

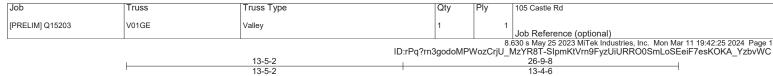
TOP CHORD 1-2=-296/209, 10-11=-251/168

NOTES

Unbalanced roof live loads have been considered for this design.

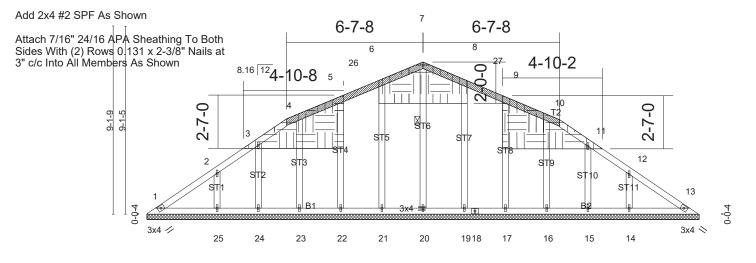
- Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=111mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C 2) Exterior (2) 0-0-4 to 3-0-4, Interior (1) 3-0-4 to 9-10-12, Exterior (2) 9-10-12 to 12-10-12, Interior (1) 12-10-12 to 19-5-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pf=35.0 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; 3) Partially Exp.; Ct=1.10
- All plates are 1.5x4 MT20 unless otherwise indicated. 4)
- Gable requires continuous bottom chord bearing. 5)
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 6)
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11, 20 except (jt=lb) 17=117, 18=118, 19=125, 15=114, 14=121, 13=113, 12=111.
- This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



Repair to Shorten Truss 1-9-0; Trusses V01GE, V02, V03, V04, V05, V17, V18, V19GE

4x4 = Scale = 1:55.9



| 0-0-6 26-9-3 | |
|---|--|
| Code Code | ES GRIP 197/144 t: 127 lb FT = 20% |

26-9-8

LUMBER-

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 OTHERS 2x4 SPF No.2 BRACING-

BOT CHORD WEBS Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt 7-20

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. All bearings 26-9-8.

(lb) - Max Horz 1=-252(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 1, 21, 22, 23, 24, 19, 17, 16, 15 except 25=-126(LC 14),

14=-124(LC 15)

Max Grav All reactions 250 lb or less at joint(s) 1, 13, 22, 24, 17, 15 except 20=257(LC 24), 21=254(LC 21), 23=252(LC 21), 25=366(LC 21), 19=251(LC 22), 16=251(LC 22), 14=359(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-25=-280/144, 12-14=-275/142

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=140mph Vasd=111mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-11 to 3-5-2, Interior(1) 3-5-2 to 13-5-2, Exterior(2) 13-5-2 to 16-5-2, Interior(1) 16-5-2 to 26-3-14 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pf=35.0 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) All plates are 1x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 21, 22, 23, 24, 19, 17, 16, 15 except (jt=lb) 25=126, 14=124.

9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



03/12/2024 4 of 4

